<u> </u>	ENERAL NOTES: THE STRUCTURAL DRAWINGS MUST BE USED IN CONJUNCTION WITH	SPECIAL LOADS:
	THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS, AND THE SPECIFICATIONS. THE CONTRACTOR MUST VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.	LIFELINE ANCHORAGE ALTERNATE #4 - FUTURE PHO SYSTEM DEAD LOAD (PENTH MAXIMUM CONSTRUCTION LO ALTERNATE #4 - FUTURE ROO SOLAR (EXISTING MAIN ROOF
2.	ALL EXISTING INFORMATION SHOWN IS REFERENCED FROM EXISTING DRAWINGS PREPARED BY: SLOAN & WHEATLEY ARCHITECTS, DATED 1/8/62.	<u>RAIN LOADS:</u> RAIN INTENSITY (15 MINUTE). <u>SNOW LOADS:</u>
3.	THE ALTERATIONS THIS STRUCTURE HAVE BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE NORTH CAROLINA STATE EXISTING BUILDING CODE, 2018 EDITION.	GROUND SNOW LOAD (Pg) FLAT ROOF LOAD (Pf) IMPORTANCE FACTOR (Is) THERMAL FACTOR (Ct) EXPOSURE FACTOR (Ce)
4.	THE WORK OUTLINED IN THE BUILDING CODE IS SUBJECT TO SPECIAL INSPECTIONS AS DESCRIBED IN THE BUILDING CODE.	DRIFT SURCHARGE (Pd) SNOW DRIF
5.	THE CONTRACTOR MUST PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE.	Pd W_0 PSF
6.	PORTIONS OF THE EXISTING STRUCTURE NOT ALTERED AND NOT AFFECTED BY THE ALTERATION HAVE NOT BEEN REVIEWED FOR COMPLIANCE WITH THE CODE REQUIREMENTS FOR A NEW STRUCTURE.	
	BEFORE PROCEEDING WITH WORK WITHIN THE EXISTING STRUCTURE, THE CONTRACTOR MUST BECOME FAMILIAR WITH THE EXISTING STRUCTURAL CONDITIONS. ANY SHORING OR BRACING SHOWN IS A PARTIAL AND SCHEMATIC REPRESENTATION OF THAT REQUIRED. THE CONTRACTOR MUST BE SOLELY RESPONSIBLE FOR THE DESIGN AND ERECTION OF ANY AND ALL SAFEGUARDS NECESSARY TO PROTECT THE EXISTING STRUCTURE. THE CONTRACTOR MUST PROVIDE SHORING, BRACING, AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF THE STRUCTURE IN A SAFE CONDITION AT ALL TIMES DURING THE PROCESS OF DEMOLITION AND	<u>NOTE:</u> SNOW DI ADDITION TO FL <u>WIND LOADS (PENTHOUSE ONLY):</u> BASIC WIND SPEED (Vult) ALLOWABLE STRESS DESIGN EXPOSURE CATEGORY INTERNAL PRESSURE COEFF <u>COMPONENT AND CLADDING</u> WALLS, ZONE 5 (10 SF) ROOF, ZONE 3 (10 SF) PARAPET, END/CORNI ULTIMATE WIND BASE SHEAF
3.	CONSTRUCTION. THE CONTRACTOR MUST FIELD VERIFY THE DIMENSIONS, ELEVATIONS, AND OTHER REQUIREMENTS NECESSARY FOR THE PROPER CONSTRUCTION AND ALIGNMENT OF THE NEW PORTIONS OF THE STRUCTURE TO THE EXISTING. ANY DIMENSIONS SHOWN OF EXISTING STRUCTURES MUST BE CONSIDERED AS APPROXIMATE AND ADEQUATE FOR BIDDING PURPOSES ONLY. THE CONTRACTOR MUST MAKE ALL MEASUREMENTS NECESSARY FOR THE FABRICATION AND ERECTION OF STRUCTURAL MEMBERS. DISCREPANCIES MUST BE BROUGHT TO THE ATTENTION OF THE ENGINEER.	Vx Vy ADDITIONAL PENTHOUSE WIN INCREASE IN THE DEMAND-CA MWFRS MEMBERS LESS THAN <u>SEISMIC LOADS:</u> SITE CLASSIFICATION. SEISMIC DESIGN CATEGORY.
).	DISCREPANCIES BETWEEN DRAWINGS, BETWEEN THE DRAWINGS AND THE SPECIFICATIONS, OR WITHIN THE SPECIFICATIONS, MUST BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER DURING THE BIDDING PROCESS IN TIME TO PERMIT CLARIFICATION BY ADDENDUM. IF INCONSISTENCIES, DISCREPANCIES OR CONTRADICTIONS IN THE CONTRACT DOCUMENTS ARE DISCOVERED AFTER THE CLOSE OF BIDDING QUESTIONS, THE CONTRACTOR MUST BE DEEMED BY SUBMITTAL OF THEIR BID, TO HAVE BID THE MOST COSTLY AS TO LABOR, MATERIALS, DURATION, SEQUENCE AND METHOD OF CONSTRUCTION TO PROVIDE THE WORK.	IMPORTANCE FACTOR (IE) <u>SPECTRAL RESPONSE ACCE</u> S _S
0.	THESE STRUCTURAL DRAWINGS ARE ISSUED ON THE DATE INDICATED FOR THE PURPOSE DESIGNATED. THESE DRAWINGS MUST NOT BE ISSUED OR RELEASED FOR ANY OTHER PURPOSE WITHOUT THE WRITTEN AUTHORIZATION OF THE STRUCTURAL ENGINEER OF RECORD.	EXISTING STRUCTURE <u>RESPONSE MODIFICATION C</u> PENTHOUSE EXISTING STRUCTURE <u>SEISMIC RESPONSE COEFFIC</u>
١.	DETAILS LABELED "TYPICAL DETAIL" WITHIN THE DOCUMENTS APPLY TO SITUATIONS ON THE PROJECT THAT MAY OCCUR THROUGHOUT THE PROJECT. SUCH DETAILS APPLY WHETHER OR NOT THE DETAIL IS SPECIFICALLY REFERENCED AT EACH INSTANCE. NOTIFY ENGINEER IF CLARIFICATIONS ARE REQUIRED REGARDING THE APPLICABILITY OF THE "TYPICAL DETAIL."	PENTHOUSE EXISTING STRUCTURE ULTIMATE SEISMIC BASE SHE PENTHOUSE EXISTING STRUCTURE ADDITIONAL PENTHOUSE SEI
2.	DESIGN CRITERIA: <u>CLASSIFICATION OF BUILDING</u> RISK CATEGORYIII	INCREASE IN THE DEMAND-C SEISMIC FORCES RESISTING 10%.
	EXISTING BUILDING CODE APPROACH - ALTERATION LEVEL III	TEMPORARY SHORIN
	SUPERIMPOSED ROOF DEAD LOADS - UNIFORM: INSULATION AND ROOF MEMBRANE 3 PSF CEILING 2 PSF SPRINKLERS 3 PSF DUCTS, LIGHTS, MISC. MECHANICAL 3 PSF SUPERIMPOSED FLOOR DEAD LOADS - UNIFORM:	1. THE CONTRACTOR MUST SUBMIT A DI WORK SEQUENCE, PREPARED BY A P REGISTERED IN THE STATE OF NORTI BRACING, AND OTHER SAFEGUARDS STRUCTURE IN A SAFE CONDITION AT PROCESS OF DEMOLITION AND CONS FROM DAMAGE THOSE PORTIONS OF
	FLOOR FINISH 3 PSF CEILING_ 2 PSF SPRINKLERS 3 PSF DUCTS, LIGHTS, MISC. MECHANICAL 3 PSF LIVE LOADS - UNIFORM:	2. GENERAL CONTRACTOR IS RESPONS SHORING AND BRACING OF THE EXIS DEMOLITION AND CONSTRUCTION UN STRUCTURAL GRAVITY AND LATERAL INCLUDES AT LOCATIONS WHERE EX
	SLAB ON GRADE100 PSFFIRST FLOOR100 PSFROOF_20 PSFCLASSROOMS40 PSFOFFICES W/ PARTITION ALLOWANCE65 PSFMECHANICAL SPACE_150 PSFLOBBIES_100 PSFCORRIDORS (FIRST FLOOR)_100 PSFCORRIDORS (SERVING PUBLIC SPACES)100 PSF	 3. BRACING MUST BE PROVIDED FOR EXARE ADJACENT TO EXISTING FLOOR S REMOVED. SHORING AND BRACING M EXISTING FLOOR SLABS THAT ARE AD SLABS INDICATED TO BE REMOVED.
	CORRIDORS (ABOVE FIRST FLOOR)80 PSF STAIRWAYS100 PSF	FOUNDATION NOTES
	LIVE LOAD REDUCTION OF THE UNIFORMLY DISTRIBUTED FLOOR LIVE LOADS HAS BEEN UTILIZED.	1. FOUNDATIONS HAVE BEEN DESIGNED RECOMMENDATIONS IN THE REPORT PREPARED BY ESP ASSOCIATES, INC
	LIVE LOADS - CONCENTRATED: STAIR TREADS300# OVER 4" SQUARE ELOOR2 000#	2. FOUNDATIONS HAVE BEEN DESIGNED BEARING PRESSURE OF 2,000 PSF.
	FLOOR2,000# ROOFS300# UNLESS OTHERWISE NOTED, CONCENTRATED LOADS ARE APPLIED UNIFORMLY OVER 2'-6" x 2'-6" AREA.	3. TOP OF FOOTING ELEVATIONS MUST BELOW LOWEST ADJACENT SOIL GRA
	UNII UNILI UVEN 2-U X 2-U AREA.	 4. PRIOR TO PLACING FOUNDATION CON EXCAVATIONS MUST BE INSPECTED E EXPLORE THE EXTENT OF LOOSE, SO UNSATISFACTORY SOIL MATERIAL AN PRESSURE. DIRECTION FOR CORREC

CONTROL GROUNDWATER AND SURF CONSTRUCTION PROCESS. INUNDATION OF BEARING SURFACES WHICH RESU BEARING MUST BE PREVENTED.

7. RETAINING WALLS HAVE BEEN DESIG

RAIN LOADS:	
RAIN INTENSITY (15 MINUTE) 6.29 IN/HR	
SNOW LOADS:GROUND SNOW LOAD (Pg)15 PSFFLAT ROOF LOAD (Pf)16.5 PSFIMPORTANCE FACTOR (Is)1.1THERMAL FACTOR (Ct)1.0EXPOSURE FACTOR (Ce)1.0	2.
	3.
Pd DRIFT VALUES	4.
₩ 0 PSF W Pd	F
① 3.7 FT 7.5 PSF	5.
<u>NOTE:</u> SNOW DRIFT LOADS ARE IN ADDITION TO FLAT ROOF LOADS. <u>WIND LOADS (PENTHOUSE ONLY):</u>	6.
BASIC WIND SPEED (Vult) 120 MPH ALLOWABLE STRESS DESIGN WIND SPEED (Vasd) 93 MPH	7.
EXPOSURE CATEGORYB INTERNAL PRESSURE COEFFICIENT±0.18 COMPONENT AND CLADDING PRESSURES:	
WALLS, ZONE 5 (10 SF) 31.6 PSF ROOF, ZONE 3 (10 SF) 65.4 PSF PARAPET, END/CORNER (10 SF) 66.7 PSF	
ULTIMATE WIND BASE SHEARS (FOR MWFRS): Vx	
Vy	С
INCREASE IN THE DEMAND-CAPACITY RATIO OF EXISTING MWFRS MEMBERS LESS THAN 10%. SEISMIC LOADS:	1.
SITE CLASSIFICATION D (ASSUMED) SEISMIC DESIGN CATEGORY B IMPORTANCE FACTOR (IE) 1.25	2.
S _S .0.155 S ₁ 0.077 S _{MS} .0.247 S _{M1} .0.184 S _{DS} .0.165 S _{D1} .0.123 ANALYSIS PROCEDURE	
(TWO-STAGE ANALYSIS) <u>LATERAL FORCE RESISTING SYSTEM:</u> PENTHOUSESTEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE	3.
EXISTING STRUCTURE REINFORCED CONCRETE MOMENT FRAMES	4.
RESPONSE MODIFICATION COEFFICIENT (R): PENTHOUSE3.0 EXISTING STRUCTUREUNCHANGED SEISMIC RESPONSE COEFFICIENT (Co);	
PENTHOUSE	5.
EXISTING STRUCTUREUNCHANGED <u>ULTIMATE SEISMIC BASE SHEAR (V):</u> PENTHOUSE10.2 KIPS	
ADDITIONAL PENTHOUSE SEISMIC LOADING RESULTS IN AN	6.
INCREASE IN THE DEMAND-CAPACITY RATIO OF EXISTING SEISMIC FORCES RESISTING SYSTEM MEMBERS LESS THAN	С
10%.	
	1.
EMPORARY SHORING/BRACING NOTE	S
THE CONTRACTOR MUST SUBMIT A DETAILED PLAN, INCLUDING WORK SEQUENCE, PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF NORTH CAROLINA FOR SHORING, BRACING, AND OTHER SAFEGUARDS TO MAINTAIN ALL PARTS OF THE STRUCTURE IN A SAFE CONDITION AT ALL TIMES DURING THE PROCESS OF DEMOLITION AND CONSTRUCTION AND TO PROTECT FROM DAMAGE THOSE PORTIONS OF THE EXISTING STRUCTURE WHICH ARE TO REMAIN. SUBMIT PLAN PRIOR TO BEGINNING WORK.	S ^{1.} 2. 3.
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_3100# (ASD) / LIFELINE

DEMOLITION NOTES:

- 1. SOME TERMS INDICATED ON PLAN ARE DEFINED AS FOLLOWS: REMOVE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND LEGALLY DISPOSE OF THEM OFF-SITE
 - REMOVE AND SALVAGE: DETACH ITEMS FROM EXISTING CONSTRUCTION AND DELIVER THEM TO THE OWNER READY FOR REUSE.
 - REMOVE AND REINSTALL: DETACH ITEMS FROM EXISTING CONSTRUCTION, PREPARE THEM FOR REUSE, AND REINSTALL THEM WHERE INDICATED.
- D. EXISTING TO REMAIN: EXISTING ITEMS OF CONSTRUCTION THAT ARE NOT TO BE REMOVED.
- COMPLY WITH LOCAL NOISE, DUST AND EROSION CONTROL REGULATIONS. CONTROL DUST FROM DEMOLITION TO PREVENT IT FROM SPREADING TO OCCUPIED PORTIONS OF BUILDING AND TO AVOID CREATING A NUISANCE IN SURROUNDING AREA.
- OBTAIN REQUIRED PERMITS FROM GOVERNING AUTHORITIES.
- PROVIDE TEMPORARY BARRICADES AND OTHER PROTECTION REQUIRED TO PREVENT INJURY TO PEOPLE AND DAMAGE TO ADJACENT PROPERTY.
- AT END OF EACH WORKDAY AND DURING INCLEMENT WEATHER. COVER AND PROTECT AREAS OF OPENED UP AND UNFINISHED WORK WITH WEATHER PROOF BARRIERS. AS REQUIRED.
- PROTECT FROM DAMAGE EXISTING ROADS, WALKS, CURBS, LANDSCAPE, AND OTHER SITE AND BUILDING STRUCTURES. REPAIR OR REPLACE DAMAGED ITEMS.
- REMOVE MATERIAL RESULTING FROM DEMOLITION OPERATIONS. EXCEPT AS OTHERWISE INDICATED, AND DISPOSE OF IN ACCORDANCE WITH APPLICABLE LAWS AND REGULATIONS AS PART OF THE WORK. CONTROL RUBBISH. DEBRIS, AND DUST BY APPROVED METHODS, AS REQUIRED BY LOCAL NOISE, DUST, AND EROSION CONTROL REGULATIONS. SOLID WASTE DISPOSAL: IN ACCORDANCE WITH COV 10.1 - 1408.1. (1997) VIRGINIA WASTE MANAGEMENT ACT; ARTICLE 2, SOLID WASTE MANAGEMENT. DISPOSAL OF SOLID WASTE IN OPEN DUMPS IS PROHIBITED.

CAST-IN-PLACE CONCRETE NOTES:

- CONCRETE MUST BE IN ACCORDANCE WITH AMERICAN CONCRETE INSTITUTE (ACI) 301 AND 318.
- CONCRETE MUST BE NORMAL WEIGHT AND MUST OBTAIN 28 DAY COMPRESSIVE STRENGTHS AS FOLLOWS: A. SLAB-ON-GRADE 3,500 PSI SUPPORTED FLOOR SLABS .4.000 PSI
- 4,500 PSI COLUMNS AND WALLS.. FOUNDATIONS 3,000 PSI CONCRETE NOT OTHERWISE NOTED. 3.000 PSI
- REINFORCING MATERIALS MUST BE AS FOLLOWS:
- REINFORCING BARS ASTM A615, GRADE 60, DEFORMED WELDED WIRE REINFORCEMENT - ASTM A1064, WELDED STEEL WIRE REINFORCEMENT; PROVIDE SHEET TYPE, ROLL TYPE IS NOT ACCEPTABLE.
- ALL REINFORCING STEEL AND EMBEDDED ITEMS SUCH AS ANCHOR RODS AND WELD PLATES MUST BE ACCURATELY PLACED AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.
- CONCRETE COVER TO REINFORCING STEEL MUST CONFORM TO THE MINIMUM COVER RECOMMENDATIONS IN ACI 318. UNLESS THE DRAWINGS SHOW GREATER COVER REQUIREMENTS.
- LAP CONTINUOUS REINFORCING STEEL PER "TYPICAL REINFORCING SPLICE SCHEDULES."

CONCRETE MASONRY NOTES:

- CONCRETE MASONRY MATERIALS AND CONSTRUCTION MUST CONFORM TO THE AMERICAN CONCRETE INSTITUTE (ACI) 530.
- CONCRETE MASONRY UNITS MUST CONFORM TO ASTM C90 AND MUST BE MADE WITH LIGHTWEIGHT AGGREGATE. MINIMUM NET AREA COMPRESSIVE STRENGTH OF MASONRY UNITS MUST BE 2,000 PSI AT 28 DAYS.
- COMPRESSIVE STRENGTH OF MASONRY MUST BE DETERMINED BY THE UNIT STRENGTH METHOD AS SET FORTH IN ACI 530.1. THE NET AREA COMPRESSIVE STRENGTH OF MASONRY, f'm, MUST BE 2,000 PSI AT 28 DAYS.
- MORTAR MUST BE TYPE 'M' OR 'S' AND MUST COMPLY WITH ASTM C270, PROPORTIONS OR PROPERTIES SPECIFICATION.
- GROUT MUST COMPLY WITH EITHER THE PROPORTIONS OR PROPERTIES SPECIFICATION OF ASTM C476 AND AS FOLLOWS: A. PROPORTIONS SPECIFICATION: THIS MIX CANNOT CONTAIN ADMIXTURES. WATER MUST BE ADDED IN THE FIELD IN ORDER TO ACHIEVE A SLUMP OF 8-11 INCHES WHEN PLACED IN THE CONCRETE MASONRY UNITS. MORTAR, PEA-GRAVEL CONCRETE, OR "CHAT" MIXES ARE NOT ACCEPTABLE SUBSTITUTES FOR THE
- SPECIFIED GROUT PROPERTIES SPECIFICATION: THIS MIX MUST BE PROPORTIONED TO OBTAIN A DOCUMENTED 28 DAY COMPRESSIVE STRENGTH OF 2,000 PSI, WITH AN 8-11 INCH SLUMP WHEN PLACED IN THE CONCRETE MASONRY UNITS.
- REINFORCING STEEL MUST COMPLY WITH ASTM A615. GRADE 60. SHOP FABRICATE REINFORCING BARS WHICH ARE SHOWN TO BE BENT OR HOOKED.
- ALL BOND BEAMS. REINFORCED CELLS AND CELLS WITH EXPANSION BOLTS, EMBED PLATES OR OTHER ANCHORS AND ALL CELLS BELOW GRADE MUST BE GROUTED SOLID. GROUT PROCEDURE MUST COMPLY WITH ACI 530.1.
- ALL CMU WALLS MUST BE REINFORCED CONTINUOUSLY FROM FOUNDATION TO TOP OF WALL. WHERE REINFORCING IS INTERRUPTED, OFFSET AND LAP ADDITIONAL BARS PER THE "TYPICAL OFFSET SPLICE AT MASONRY WALL DETAILS."
- LAP ALL REINFORCING PER SCHEDULE BELOW, TYPICAL UNLESS OTHERWISE NOTED:

MASONRY LAP SCHEDULE 72 x BAR DIAMETER **REINF SIZE** 36" #4 #5 45" #6 54" #7 63" 72" #8

CONCRETE MASONRY NOTES:

- 13. PROVIDE STANDARD 9 GAGE LADDER TYPE HORIZONTAL JOINT REINFORCING IN CMU WALLS AT 16 INCHES ON CENTER AND IN TWO JOINTS IMMEDIATELY ABOVE AND BELOW ALL OPENINGS, EXTENDING A MINIMUM OF 2 FEET BEYOND THE JAMB ON EACH SIDE OF THE OPENING, EXCEPT AT CONTROL JOINTS.
- 14. PROVIDE HORIZONTAL BOND BEAMS WITH CONTINUOUS REINFORCING AS SHOWN IN THE SECTIONS AND DETAILS DISCONTINUE ALL HORIZONTAL REINFORCING AT CONTROL JOINTS.
- 15. DO NOT LOCATE CONTROL JOINTS WITHIN TWO FEET OF STEEL BEAM BEARING LOCATIONS.

STRUCTURAL STEEL NOTES:

- 2. STRUCTURAL STEEL MUST COMPLY WITH THE FOLLOWING SPECIFICATIONS: A. STRUCTURAL STEEL SHAPES, PLATES AND BARS UNLESS
 - OTHERWISE NOTED ASTM A36, Fy = 36 KSI STRUCTURAL STEEL W-SHAPES - ASTM A992, Fy = 50 KSI

 - D. ANCHOR RODS ASTM F1554, GRADE 36
 - FULLY PRETENSIONED BOLTS ASTM F1852 (TWIST-OFF TYPE) G. WASHERS - ASTM F436
 - H. NUTS ASTM A563
- BELOW.

MINIMUM BEAM REACTION SCHEDULE

BEAM SIZE	DESIGN REACTION (LRFD)	MIN # OF BOLTS		
W8	15 KIPS	2		
W10	25 KIPS	2		
W12	35 KIPS	3		
W21	40 KIPS	5		

- "DELEGATED DESIGN" CONNECTIONS.
- THIS REVIEW AND VERIFICATION HAS BEEN COMPLETED.
- SHEAR CONNECTIONS MOMENT CONNECTIONS C. HSS BEAM CONNECTIONS
- FOLLOWS:
- JOIST/BEAM SPACIN TO 6'-0"

10. PROVIDE MECHANICAL SPLICES FOR ALL #9 BARS AND LARGER AND FOR ALL BARS AT CONTRACTOR'S OPTION.

11. ALL NON-BEARING MASONRY WALLS MUST BE REINFORCED WITH #4 VERTICAL BARS AT 40 INCHES ON CENTER, TYPICAL UNLESS OTHERWISE NOTED. ALL NON-BEARING MASONRY WALLS MUST BE BRACED PER "TYPICAL NON-BEARING MASONRY PARTITION DETAILS."

12. PROVIDE REINFORCING STEEL DOWELS OF THE SAME SIZE AND SPACING AS VERTICAL REINFORCING FROM THE SUPPORTING STRUCTURE. DOWELS MUST HAVE STANDARD ACI HOOKS.

1. STRUCTURAL STEEL MUST BE IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC) 360.

- HOLLOW STRUCTURAL SECTIONS (HSS)
- a. SQUARE & RECTANGULAR ASTM A500, GRADE C, Fy = 50 KSI
- HIGH STRENGTH BOLTS ASTM A325 (TYPICAL UON)

3. UNLESS OTHERWISE NOTED, ALL REQUIRED DESIGN STRENGTHS AND REACTIONS INDICATED ARE BASED ON THE "LOADING COMBINATIONS USING STRENGTH DESIGN OR LOAD AND RESISTANCE FACTOR DESIGN" PER SECTION 1605.2 OF THE BUILDING CODE

4. UNLESS OTHERWISE NOTED, BEAM CONNECTIONS MUST BE AISC "SIMPLE SHEAR CONNECTIONS" WITH ASTM A325 BOLTS. DESIGN CONNECTIONS FOR THE REACTIONS (LRFD FACTORED LOADING) SHOWN ON THE DRAWINGS AND THE MINIMUM NUMBER OF BOLTS SHOWN BELOW. IF NO REACTION IS SHOWN, DESIGN CONNECTIONS FOR REACTIONS AND THE MINIMUM NUMBER OF BOLTS SHOWN

5. HIGH STRENGTH BOLTS MAY BE TIGHTENED TO THE "SNUG TIGHT" CONDITION IN LIEU OF FULL PRETENSIONING EXCEPT FOR THE FOLLOWING CONNECTIONS WHICH MUST BE FULLY PRETENSIONED: A. DIRECT TENSION CONNECTIONS IDENTIFIED AS (DT) ON PLAN. BOLTED CONNECTIONS USING NON-STANDARD HOLES.

6. REFER TO THE SPECIFICATIONS FOR REQUIREMENTS OF

7. FOR STRUCTURAL STEEL CONNECTIONS INDICATED AS "DELEGATED DESIGN", INCLUDE STRUCTURAL CALCULATIONS SIGNED AND SEALED BY THE QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF NORTH CAROLINA RESPONSIBLE FOR THEIR PREPARATION. IN ADDITION, THE PROFESSIONAL ENGINEER RESPONSIBLE FOR CONNECTION DESIGN MUST REVIEW THE SHOP DRAWINGS PRIOR TO SUBMITTAL TO VERIFY THAT THE CONNECTIONS AS DETAILED ON THE SHOP DRAWINGS COMPLY WITH THE CONNECTION DESIGN REQUIREMENTS OF THE FINAL CALCULATIONS. A REVIEW LETTER. SIGNED AND SEALED BY THE PROFESSIONAL ENGINEER RESPONSIBLE FOR CONNECTION DESIGN MUST BE PROVIDED WITH THE SHOP DRAWINGS AND CALCULATION SUBMITTAL STATING THAT

DELEGATED DESIGN CONNECTIONS ARE AS FOLLOWS:

9. HIGH STRENGTH BOLTS MUST BE FULLY PRETENSIONED USING TENSION CONTROL "TWIST OFF" BOLTS.

10. PROVIDE ANGLE FRAMING AROUND OPENINGS LARGER THAN 6 INCHES IN ANY DIMENSION (INCLUDING ROOF DRAINS) TO SUPPORT STEEL DECK, TYPICAL UNLESS OTHERWISE NOTED OR DETAILED AS

١G	ANGLE SIZE
	L3x3x1/4

STRUCTURAL STEEL NOTES:

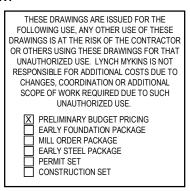
- 11. WELDING MUST BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL." WELD ELECTRODES MUST BE E70XX LOW HYDROGEN. UNLESS OTHERWISE NOTED, PROVIDE CONTINUOUS FILLET WELDS WITH MINIMUM SIZE REQUIRED BY TABLE J2.4 AISC 360.
- 12. COORDINATE ALL MEMBER LOCATIONS, UNIT WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED.
- 13. STRUCTURAL STEEL SCHEDULED TO RECEIVE SPRAYED-ON FIREPROOFING MUST NOT BE PRIME PAINTED.
- 14. HOT-DIP GALVANIZE AFTER FABRICATION THE FOLLOWING: A. ANGLES AND PLATES SUPPORTING MASONRY IN EXTERIOR WALLS.
- B. LINTELS AND LINTEL ASSEMBLIES SUPPORTING MASONRY IN EXTERIOR WALLS C. ALL STEEL EXPOSED TO WEATHER IN THE FINAL CONSTRUCTION.
- D. ITEMS IDENTIFIED AS GALVANIZED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS.
- 15. ALL MEMBERS EXPOSED TO VIEW IN THE FINISHED CONSTRUCTION MUST BE CONSIDERED ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS).
- 16. STEEL MEMBERS MUST BE SPLICED ONLY WHERE INDICATED. CONTINUOUS MEMBERS MUST BE SPLICED OVER SUPPORTS, UNLESS OTHERWISE NOTED. MEMBERS INDICATED AS DIAPHRAGM CHORDS (DC) MUST HAVE FULL PENETRATION BUTT WELD SPLICES, UNLESS OTHERWISE NOTED.

STEEL DECK NOTES:

- 1. STEEL DECK MUST BE IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI), "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS" AND THE STEEL DECK INSTITUTE (SDI), "DESIGN MANUAL FOR COMPOSITE DECKS, FORM DECKS, AND ROOF DECKS."
- 2. STEEL DECK INSTALLATION MUST COMPLY WITH THE FOLLOWING: ROOF DECK (UNLESS OTHERWISE NOTED): 1 1/2" x 22 GAGE TYPE 'B' GALVANIZED. UNLESS OTHERWISE NOTED, ATTACH DECK TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS IN ALL RIBS WHERE END LAPS OCCUR AND AT 12 INCHES ON CENTER ALONG SUPPORTS WITH A 36/4 PATTERN. FASTEN SIDE LAPS WITH #10 SELF-TAPPING HEX HEAD SCREWS AT 1/5 POINTS BETWEEN SUPPORTS. FASTEN EDGEMOST DECK PANEL TO STEEL FRAMING WITH 5/8 INCH DIAMETER PUDDLE WELDS AT SAME SPACING AS SIDELAP FASTENERS
- ROOF DECK (AS INDICATED): 3" x 20 GAGE TYPE 'N' GALVANIZED. R UNLESS OTHERWISE NOTED, ATTACH DECK TO STEEL SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS IN ALL RIBS WHERE END LAPS OCCUR AND AT 8 INCHES ON CENTER ALONG SUPPORTS WITH A 24/4 PATTERN. FASTEN SIDE LAPS WITH #10 SELF-TAPPING HEX HEAD SCREWS AT 1/8 POINTS BETWEEN SUPPORTS. FASTEN EDGEMOST DECK PANEL TO STEEL FRAMING WITH 5/8 INCH DIAMETER PUDDLE WELDS AT SAME SPACING AS SIDELAP FASTENERS.
- FORM DECK: 1 1/2" x 20 GAGE TYPE 'C' GALVANIZED. UNLESS OTHERWISE NOTED, ATTACH DECK TO SUPPORTS WITH 5/8 INCH DIAMETER PUDDLE WELDS AT 12 INCHES ON CENTER. FASTEN SIDELAPS WITH #10 SELF-TAPPING HEX HEAD SCREWS AT 1/4 POINTS BETWEEN SUPPORTS. FASTEN EDGEMOST DECK PANEL TO STEEL FRAMING WITH 5/8 INCH DIAMETER PUDDLE WELDS AT SAME SPACING AS SIDELAP FASTENERS.
- 3. STEEL DECK MUST BE INSTALLED PERPENDICULAR TO SUPPORTS AND MUST HAVE A MINIMUM OF THREE CONTINUOUS SPANS. ENDLAPS MUST ONLY OCCUR AT SUPPORTS.
- 4. WELDING MUST BE IN ACCORDANCE WITH AWS D1.3 "STRUCTURAL WELDING CODE - SHEET STEEL."
- 5. PERMANENT SUSPENDED LOADS MUST NOT BE SUPPORTED BY STEEL ROOF DECK.
- 6. STEEL DECK SCHEDULED TO RECEIVE SPRAYED-ON FIREPROOFING MUST BE GALVANIZED.
- 7. CONDUIT AND PIPING MUST NOT BE PLACED IN ELEVATED SLABS.

COLD-FORMED METAL FRAMING NOTES:

- COLD-FORMED METAL FRAMING MUST BE IN ACCORDANCE WITH THE AMERICAN IRON AND STEEL INSTITUTE (AISI) "NORTH AMERICAN STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS."
- SUBMIT SHOP DRAWINGS AND CALCULATIONS SIGNED AND SEALED BY A NORTH CAROLINA LICENSED PROFESSIONAL ENGINEER RESPONSIBLE FOR THE DESIGN OF INTERIOR AND EXTERIOR NON-LOAD BEARING COLD-FORMED METAL FRAMING. SHOP DRAWINGS MUST INCLUDE DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE. INCLUDE PLACING DRAWINGS FOR FRAMING MEMBERS SHOWING SIZE AND GAGE DESIGNATIONS, NUMBER, TYPE, LOCATION AND SPACING. INDICATE CONNECTIONS, SUPPLEMENTAL STRAPPING, BRACING, SPLICES, BRIDGING, ACCESSORIES AND DETAILS AND CONSTRUCTION SEQUENCE REQUIRED FOR PROPER AND SAFE INSTALLATION.
- WELDING MUST BE IN ACCORDANCE WITH AWS D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL." TOUCH UP ALL WELDS WITH SPECIFIED COATING SYSTEMS
- 4. COLD-FORMED METAL FRAMING MEMBERS MUST CONFORM TO ASTM C955, AND BE FORMED OF CORROSION-RESISTANT STEEL CONFORMING TO ASTM A653 AND ASTM C955 WITH A MINIMUM YIELD STRENGTH OF 33 KSI FOR 43 MIL AND THINNER MEMBERS AND 50 KSI FOR ALL OTHER MEMBERS.
- MEMBER SECTION PROPERTIES MUST CONFORM TO PART 'I' OF THE "COLD-FORMED STEEL DESIGN MANUAL."
- 6. COLD-FORMED METAL FRAMING MEMBERS, HEADERS AND CONNECTIONS SHOWN ON STRUCTURAL AND ARCHITECTURAL DRAWINGS ARE SCHEMATIC ONLY AND MUST BE DESIGNED TO MEET PERFORMANCE SPECIFICATION REQUIREMENTS.
- 7. PROVIDE BRIDGING LINES AT 4'-0" MAXIMUM ON CENTER IN ALL WALLS UNLESS OTHERWISE INDICATED. BRIDGING MUST BE FULLY INSTALLED AND ANCHORED AT ENDS BEFORE SUPERIMPOSING LOADS ONTO THE STUDS.
- 8. SCREWS ARE SELF DRILLING SCREWS (SDS). MINIMUM SCREW SPACING AND EDGE DISTANCE MUST BE 3/4" IN ANY DIRECTION, TYPICAL.
- 9. POWDER ACTUATED FASTENERS (PAF) MUST HAVE A MINIMUM ALLOWABLE CAPACITY INTO THE BASE MATERIAL AS FOLLOWS, UNLESS OTHERWISE NOTED: A. STEEL: SHEAR = 600 LBS: TENSION = 250 LBS
- B. CONCRETE: SHEAR = 260 LBS: TENSION = 255 LBS
- 10. LOW PROFILE SCREWS MUST BE USED AS REQUIRED FOR ARCHITECTURAL REQUIREMENTS.





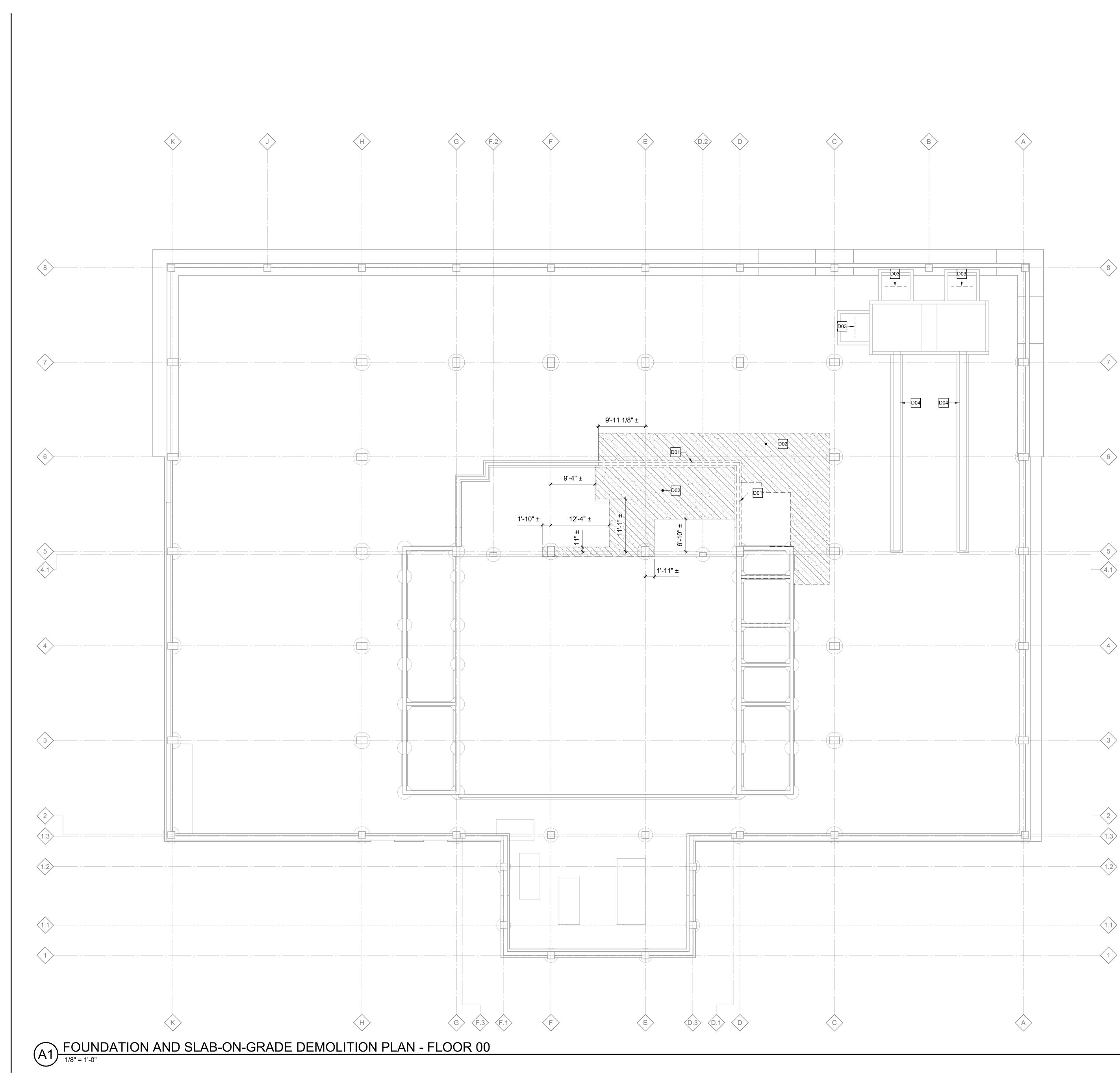
SPECIALTY STRUCTURAL ELEMENTS:		DRAWINGS LEGEND	
. THE FOLLOWING BUILDING ELEMENTS REQUIRE DELEGATED DESIGN AND ENGINEERING BY A SPECIALTY STRUCTURAL ENGINEER:	GENERAL ANNOTATIONS	ELEVATIONS	CONCRETE
AND ENGINEERING BY A SPECIALLY STRUCTURAL ENGINEER. A. METAL STAIRS B. CURTAIN WALL AND GLAZING ASSEMBLIES INCLUDING	SECTIONS	FOUNDATIONS	WALLS
CONNECTIONS TO THE STRUCTURE C. INTERIOR AND EXTERIOR NON-LOAD BEARING COLD-FORMED	SECTION/DETAIL NUMBER/LETTER	(-X'-X") = TOP OF FOOTING ELEVATION (-X'-X") = MEASURED FROM REFERENCED	BEARING WALL EXTENDING
METAL FRAMING (CFMF) D. STRUCTURAL STEEL CONNECTIONS	= SECTION/DETAIL MARK	FINISHED FLOOR ELEVATION = 0'-0"	
E. PRE-FABRICATED CANOPIES AND AWNINGS F. TEMPORARY SHORING AND/OR EXCAVATION SUPPORT	SHEET NUMBER WHERE SECTION/DETAIL MARK IS DRAWN	(-X'-X") = TOP OF PILE CAP / GRADE BEAM ELEVATION MEASURED FROM	BELOW FLOOR / ROOF
G. MECHANICAL, ELECTRICAL, AND PLUMBING SUPPORTS AND DISTRIBUTIONS SYSTEMS, INCLUDING BRACING AND	SECTION/DETAIL NUMBER/LETTER	REFERENCED FINISHED FLOOR ELEVATION = 0'-0"	
ATTACHMENTS. REFERENCE SPECIFICATIONS FOR COMPLETE	= SECTION/DETAIL MARK	TOP OF EXISITNG FOOTING <-X'-X"> = ELEVATION MEASURED FROM	= CONCRETE SHEARWALL = CONCRETE FIREWALL
REQUIREMENTS	SHEET NUMBER WHERE SECTION/DETAIL MARK IS DRAWN	REFERENCED FINISHED FLOOR ELEVATION = 0'-0"	$\langle w_1 \rangle$ = WALL TYPE MARK
IT COMPLETE CALCULATIONS AND SHOP DRAWINGS, SIGNED EALED BY THE PROFESSIONAL ENGINEER REGISTERED IN THE	SHEET NUMBER WHERE	X'-X" TOP OF SLAB ELEVATION = MEASURED FROM REFERENCED	
E OF NORTH CAROLINA RESPONSIBLE FOR THE DESIGN, JDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE		FINISHED FLOOR ELEVATION = 0'-0"	MASONRY
PORTING STRUCTURE. INCLUDE A SUMMARY OF THE TROLLING LOAD CASES FOR EACH LOCATION.		FLOORS AND ROOF	WALLS
DITION TO THEIR OWN DEAD WEIGHT AND THE DEAD LOADS VN OR INDICATED IN THE DRAWINGS, MEMBERS MUST BE	GRID = COLUMN GRID MARK		
TO SUPPORT THE LOADS INDICATED IN THE GENERAL	= EXISTING COLUMN GRID MARK	BOD = +X'-X" = MEASURED FROM REFERENCED FINISHED FLOOR ELEVATION = 0'-0"	$= \frac{1}{1} = $
TION DETAILS SHOWN ARE SCHEMATIC ONLY. ALL		TOP OF STEEL ELEVATION TOS = +X'-X" = MEASURED FROM REFERENCED	BELOW FLOOR / ROOF
TIONS MUST BE DESIGNED AND DETAILED BY THE CTURER TO SUIT THE SPECIFIED LOADS. CONNECTIONS	GENERAL PLANS	FINISHED FLOOR ELEVATION = 0'-0"	= NON-BEARING WALL BEARI FLOOR BELOW
OUNT FOR THERMAL MOVEMENT, DEFLECTION AND CREEP. . CONNECTIONS ON SHOP DRAWINGS.	X = PLAN KEY NOTE MARK	BOTTOM OF STEEL ELEVATION BOS = +X'-X" = MEASURED FROM REFERENCED FINISHED FLOOR ELEVATION = 0'-0"	EXTERIOR WALL TERMINAT BELOW FLOOR / ROOF
TRACTOR MUST BE RESPONSIBLE FOR THE COORDINATION PECIALTY STRUCTURAL ELEMENTS AND COST ASSOCIATED	± = FIELD VERIFY	TOP OF MASONRY ELEVATION	= CMU SHEARWALL
ONTRACTOR INITIATED CHANGE IN BUILDING STRUCTURE, G CONSTRUCTION COSTS AND RE-ENGINEERING COSTS.	SL = DIRECTION OF SLOPE	TOM = +X'-X" = MEASURED FROM REFERENCED FINISHED FLOOR ELEVATION = 0'-0"	= CMU FIREWALL
		TRUSS BEARING ELEVATION	W1 = WALL TYPE MARK
INSTALLED ANCHOR NOTES:	= CHANGE IN ELEVATION	TBE = +X'-X"=MEASURED FROM REFERENCEDFINISHED FLOOR ELEVATION = 0'-0"	STEEL LINTELS
T INSTALLED ANCHORS INDICATED ON THE DRAWINGS ARE INC, AND MUST BE CONSIDERED THE BASIS OF DESIGN	= CHANGE IN SLOPE		
, INC, AND MOST BE CONSIDERED THE BASIS OF DESIGN CT. WHERE NOT EXPLICITLY INDICATED IN THE DRAWINGS, LOWING ANCHORS/ADHESIVES MUST BE USED:		STEEL	L-X = STEEL LINTEL MARK
CHORAGE TO CONCRETE	SHALLOW FOUNDATIONS	CONNECTIONS	A = STEEL LINTEL BEARING PLA
CONCRETE USE: a. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW	SF Î SF = PIPE CROSSING FOOTING	T►→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→→	
DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM SYSTEM (VC 20-U OR VC40U) WITH STEEL THREADED		T	
ROD PER ICC ESR-3187. CREW ANCHORS FOR CRACKED AND UNCRACKED ONCRETE USE:	= SLAB-ON-GRADE JOINT	Image: Image	
HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR-3027. R DOWELING INTO CONCRETE	WFX = WALL FOOTING MARK	$T \longrightarrow = JOIST BOTTOM CHORD EXTENSION$	
DHESIVE ANCHORS FOR CRACKED AND UNCRACKED			
. HILTI HIT-HY 200 SAFE SET SYSTEM WITH HILTI HOLLOW DRILL BIT (TE-CD OR TE-YD) AND VC 20/40 VACUUM	CFX = COLUMN FOOTING MARK	$ \left \begin{array}{c} \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\ \\$	
SYSTEM (VC 20-U OR VC 40-U) WITH CONTINUOUSLY DEFORMED REBAR PER ICC ESR-3187.	SHALLOW FOUNDATIONS		
GE TO SOLID GROUTED MASONRY IVE ANCHORS USE: _TI HIT-HY 270 MASONRY ADHESIVE ANCHORING	PCX = PILE CAP MARK	KB = KNUCKLED BEAM	
SYSTEM (ICC PENDING). STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E	GBX = GRADE BEAM MARK	MISC ANNOTATIONS	
CONTINUOUSLY THREADED ROD. ECHANICAL ANCHORS USE:			
A. HILTI KWIK HUS EZ SCREW ANCHORS PER ICC ESR 3056. HORAGE TO HOLLOW / MULTI-WYTHE MASONRY	TPX = TEST PILE MARK	(SJ) = JOIST SLIP CONNECTION	
DHESIVE ANCHORS USE: . HILTI HIT-HY 270 MASONRY ADHESIVE ANCHORING	FLOOR AND ROOF FRAMING	(SG) = JOIST GIRDER SLIP CONNECTION	
SYSTEM PERICCESR-3342. STEEL ANCHOR ELEMENT MUST BE HILTI HAS-E	MECHANICAL UNIT SUPPORTED	(SB) = BEAM SLIP CONNECTION	
CONTINUOUSLY THREADED ROD OR CONTINUOUSLY DEFORMED STEEL REBAR. THE APPROPRIATE SIZE SCREEN TUBE MUST BE USED	ABOVE FRAMING (WEIGHT IN XXXX# POUNDS) - COORD W/ MECH DWGS	JOIST GIRDER MOMENT MARK	
PER ADHESIVE MANUFACTURER'S RECOMMENDATION.	RTU-X XXXX#MECHANICAL UNIT SUPPORTED BELOW FRAMING (WEIGHT IN		
E POST INSTALLED ANCHOR PRODUCTS MAY BE TO THE ENGINEER FOR REVIEW AND POSSIBLE	POUNDS) - COORD W/ MECH DWGS		
VAL. ALL SUBSTITUTION REQUESTS MUST BE ACCOMPANIED CC ESR SHOWING COMPLIANCE WITH THE RELEVANT	= FLOOR / ROOF OPENING		
GODE FOR SEISMIC USES, LOAD RESISTANCE, ATION CATEGORY, AND COMPREHENSIVE INSTALLATION TIONS, ADDESING ANGLIOD EVALUATION WILL ALSO			
CTIONS. ADHESIVE ANCHOR EVALUATION WILL ALSO ER CREEP, IN-SERVICE TEMPERATURE AND INSTALLATION RATURE. ALTERNATE PRODUCTS MAY REQUIRE	= WARP LINE OF ROOF DECK		
CATIONS TO ANCHOR DIAMETER, SPACING, AND EMBEDMENT.	= HORIZONTAL BRIDGING		
ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS ED IN THE ANCHOR PACKAGING.	= CROSS BRIDGING		
NTRACTOR MUST ARRANGE FOR AN ANCHOR	HIGH SIDE OF BRACE		
CTURER'S REPRESENTATIVE TO PROVIDE ONSITE ATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS	= BEAM BOTTOM FLANGE BRACE		
D. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE ITED CONFIRMATION THAT ALL OF THE CONTRACTOR'S	LOW SIDE OF BRACE		
L WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE EMENT OF ANCHOR INSTALLATION.		ے۔ 	
R CAPACITY IS DEPENDANT UPON SPACING BETWEEN INT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF			
E. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EARANCES INDICATED ON THE DRAWINGS.			
REINFORCING BARS IN THE CONCRETE STRUCTURE MAY			
CT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON AWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR			
CATE THE POSITION OF THE REINFORCING BARS AT THE NS OF THE CONCRETE ANCHORS, BY FERROSCAN OR GPR.			
ST INSTALLED ANCHORS REQUIRE CONTINUOUS SPECIAL TIONS TO VERIFY INSTALLATION HAS BEEN PERFORMED IN			
TIONS TO VERIFY INSTALLATION HAS BEEN PERFORMED IN RDANCE WITH THE MANUFACTURER'S WRITTEN INSTRUCTIONS. ENCE THE STATEMENT AND SCHEDULE OF SPECIAL			
CTIONS FOR ADDITIONAL INFORMATION.			

		CONCRETE
		WALLS
Т. Т	=	BEARING WALL EXTENDING ABOVE FLOOR / ROOF
	=	BEARING WALL TERMINATING BELOW FLOOR / ROOF
	=	EXTERIOR WALL TERMINATING BELOW FLOOR / ROOF
	=	CONCRETE SHEARWALL
	=	CONCRETE FIREWALL
	=	WALL TYPE MARK
		MASONRY
		WALLS
TA T	=	BEARING WALL EXTENDING ABOVE FLOOR / ROOF
\downarrow	=	BEARING WALL TERMINATING BELOW FLOOR / ROOF
K	=	NON-BEARING WALL BEARING ON FLOOR BELOW
	=	EXTERIOR WALL TERMINATING BELOW FLOOR / ROOF
	=	CMU SHEARWALL
	=	CMU FIREWALL
	=	WALL TYPE MARK
		STEEL LINTELS
	=	STEEL LINTEL MARK

ABBREVIATIONS

AFF	ABOVE FINISHED FLOOR	KCJ	KEYED CONSTRUCTION JOINT
ARCH	ARCHITECT	Ld	REBAR TENSION
BD	BAR DIAMETER		DEVELOPMENT LENGTH
BF	BRACED FRAME	Ldc	REBAR COMPRESSION
BEJ	BUILDING EXPANSION JOINT	200	DEVELOPMENT LENGTH
BLDG	BUILDING	Ldh	HOOKED REBAR TENSION
BM	BEAM	Lan	DEVELOPMENT LENGTH
BOD	BOTTOM OF DECK	Ls	REBAR TENSION SPLICE
BOS	BOTTOM OF STEEL	LO	LENGTH
BOT, B	BOTTOM	Lsc	REBAR COMPRESSION SPLICE
BRG	BEARING	230	LENGTH
BTWN	BETWEEN	L	LOW
C TO C	CENTER TO CENTER		LONG LEG HORIZONTAL
CFMF	COLD-FORMED METAL	LLV	LONG LEG VERTICAL
	FRAMING	LSH	LONG SIDE HORIZONTAL
CJ	CONTROL JOINT	LSV	LONG SIDE VERTICAL
CL	CENTERLINE	LTWT	LIGHTWEIGHT
CLR	CLEAR	LWC	LIGHTWEIGHT CONCRETE
CMU	CONCRETE MASONRY UNIT	MAS	MASONRY
COL	COLUMN	MATL	MATERIAL
CONC	CONCRETE	MAX	MAXIMUM
CONN	CONNECTION	MECH	MECHANICAL
CONSTR	CONSTRUCTION	MECH	MOMENT FRAME
CONT	CONTINUOUS	MFR	MANUFACTURER
COORD	COORDINATE	MID	MIDDLE
CTR	CENTER	MIN	MINIMUM
CTRD	CENTERED	MOD	MODIFY
DBA	DEFORMED BAR ANCHOR	MOD	MIDDEPTH OF SLAB
DBL	DOUBLE	NOM	NOMINAL
DC		NS	NEAR SIDE
	DIAPHRAGM CHORD		NOT TO SCALE
DCJ	DOWELED CONSTRUCTION		ON CENTER
	JOINT DIAMETER		OPPOSITE HAND
			OPENING
		OPNG	
DJ		PAF	POWDER ACTUATED
DWGS	DRAWINGS		FASTENER
EA	EACH	PAR	PARALLEL
		PEMB	PRE-ENGINEERED METAL
	ELEVATION		BUILDING
		PEN	PENETRATE, PENETRATION
EMBED	EMBEDMENT		PERPENDICULAR
EOD		PL	PLATE
		PT	POST-TENSIONED (CONC)
EQ		-	PRESSURE TREATED (WOOD)
	EACH WAY	R	RADIUS
		REF	REFERENCE, REFER TO
		REINF	REINFORCE, REINFORCED,
	EXTERIOR		REINFORCING
	FLOOR DRAIN	REQD	
			REQUIREMENTS
FO			SCHEDULE
FFEL	FINISHED FLOOR		STEPPED FOOTING
			STEPPED GRADE BEAM
			SIMILAR
		SJ	
	FACE OF BUILDING	SL	
	FACE OF CONCRETE FACE OF MASONRY		SLAB-ON-GRADE
			SIDEPLATE FRAME STANDARD
			STIFFENER
			TRUSS BEARING ELEVATION
	FAR SIDE		
		T&B T&G	TOP & BOTTOM TONGUE AND GROOVE
	FIELD VERIFY GALVANIZED		THICKNESS
	GENERAL CONTRACTOR	THK TOC	TOP OF CONCRETE
GEN	GENERAL CONTRACTOR GENERAL		TOP OF CONCRETE TOP OF FOOTING
GR BM H	GRADE BEAM HIGH		TOP OF MASONRY TOP OF CONCRETE PEDESTAL
hk Horiz	HOOK HORIZONTAL	TOS TS	TOP OF STEEL THICKENED SLAB
HORIZ			THICKENED SLAB THICKENED SLAB AT STAIR
100	SECTION	-	TYPICAL
ПСЛ	HEADED STUD ANCHOR	TYP	
HSA HST BM			UNLESS OTHERWISE NOTED
HT	HEIGHT	VERT W/	WITH
HVY	HEAVY	WP	WITH WORKING POINT
INT JBE			WOOD STRUCTURAL PANEL(S) WELDED WIRE REINFORCING
JDE	JOINT		
KCJ	KEYED CONSTRUCTION		
	JOINT		





FOUNDATION PLAN NOTES

- A. REFERENCE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
- B. ACTUAL FINISHED FLOOR ELEVATIONS ARE SPECIFIED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIAL.
- C. TOP OF ALL FOOTINGS ARE INDICATED ON PLAN.
- D. NOT ALL UTILITY LOCATIONS ARE SHOWN ON PLAN. THE CONTRACTOR MUST COORDINATE THE LOCATIONS, SIZES, AND INVERTS OF UTILITIES. AT LOCATIONS WHERE UTILITIES PASS BELOW THE TOP OF FOOTING ELEVATION, STEP THE TOP OF FOOTING DOWN ON EACH SIDE PER THE "TYPICAL STEPPED FOOTING DETAIL" AND SLEEVE THE UTILITY THROUGH THE FOUNDATION WALL. THE CONTRACTOR MAY, AT HIS/HER OPTION, SLEEVE THE UTILITY THROUGH THE FOUNDATION PER THE "TYPICAL PIPE SLEEVE AT WALL FOOTING DETAILS."
- E. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 6 INCHES BEYOND ENDS OF WALLS.
- F. SITE WALLS ARE NOT SHOWN ON PLAN. CONTRACTOR MUST COORDINATE CIVIL AND LANDSCAPE DRAWINGS FOR SITE WALL INFORMATION.
- G. DIMENSIONS SHOWN ON FOUNDATION PLAN ARE TO COLUMN GRIDLINES AND OUTSIDE FACE OF FOUNDATION WALLS, UNLESS OTHERWISE NOTED.

SLAB/DECK PLAN NOTES

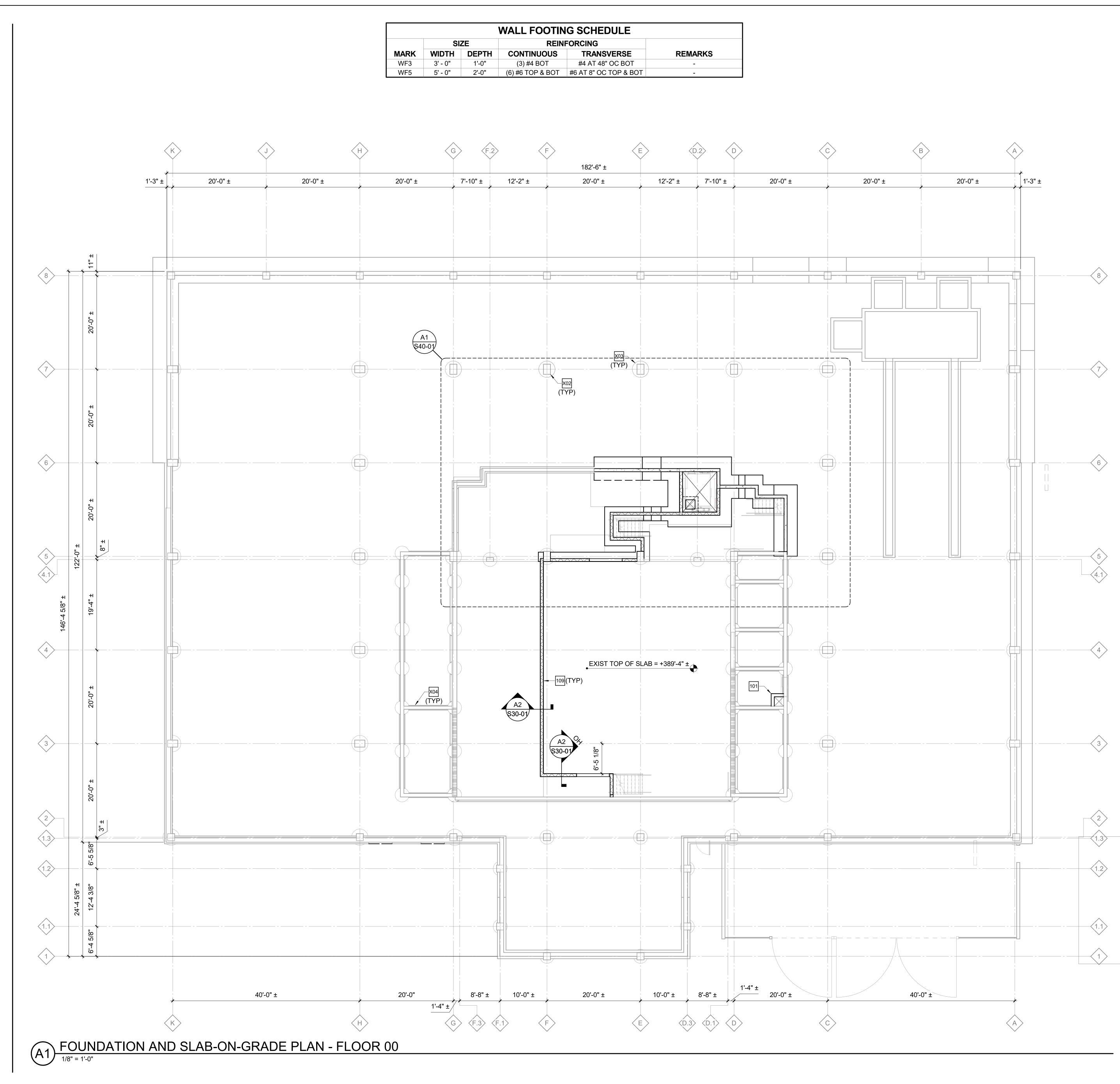
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- G. PLACE (1) #4 x 3'-0" IN MIDDEPTH OF SLAB AT RE-ENTRANT CORNERS WHERE A SLAB JOINT DOES NOT OCCUR.
- H. EXISTING SLAB-ON-GRADE IS 4" CONCRETE, UNLESS OTHERWISE NOTED.

KEY NOTES

D01	EXISTING WALL AND WALL FOOTING TO BE DEMOLISHED.
D02	EXISTING SLAB TO BE DEMOLISHED.
D03	EXISTING STEEL BEAM TO BE DEMOLISHED.
D04	EXISTING COVER PLATE OVER TRENCH TO BE

EXISTING COVER PLATE OVER TRENCH TO BE DEMOLISHED.





	WALL FOOTING SCHEDULE					
SIZE		ZE	REINFORCING			
	WIDTH	DEPTH	CONTINUOUS	TRANSVERSE	REMARKS	
	3' - 0"	1'-0"	(3) #4 BOT	#4 AT 48" OC BOT	-	
	5' - 0"	2'-0"	(6) #6 TOP & BOT	#6 AT 8" OC TOP & BOT	-	

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- E. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 6 INCHES BEYOND ENDS OF WALLS.
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SLAB/DECK PLAN NOTES

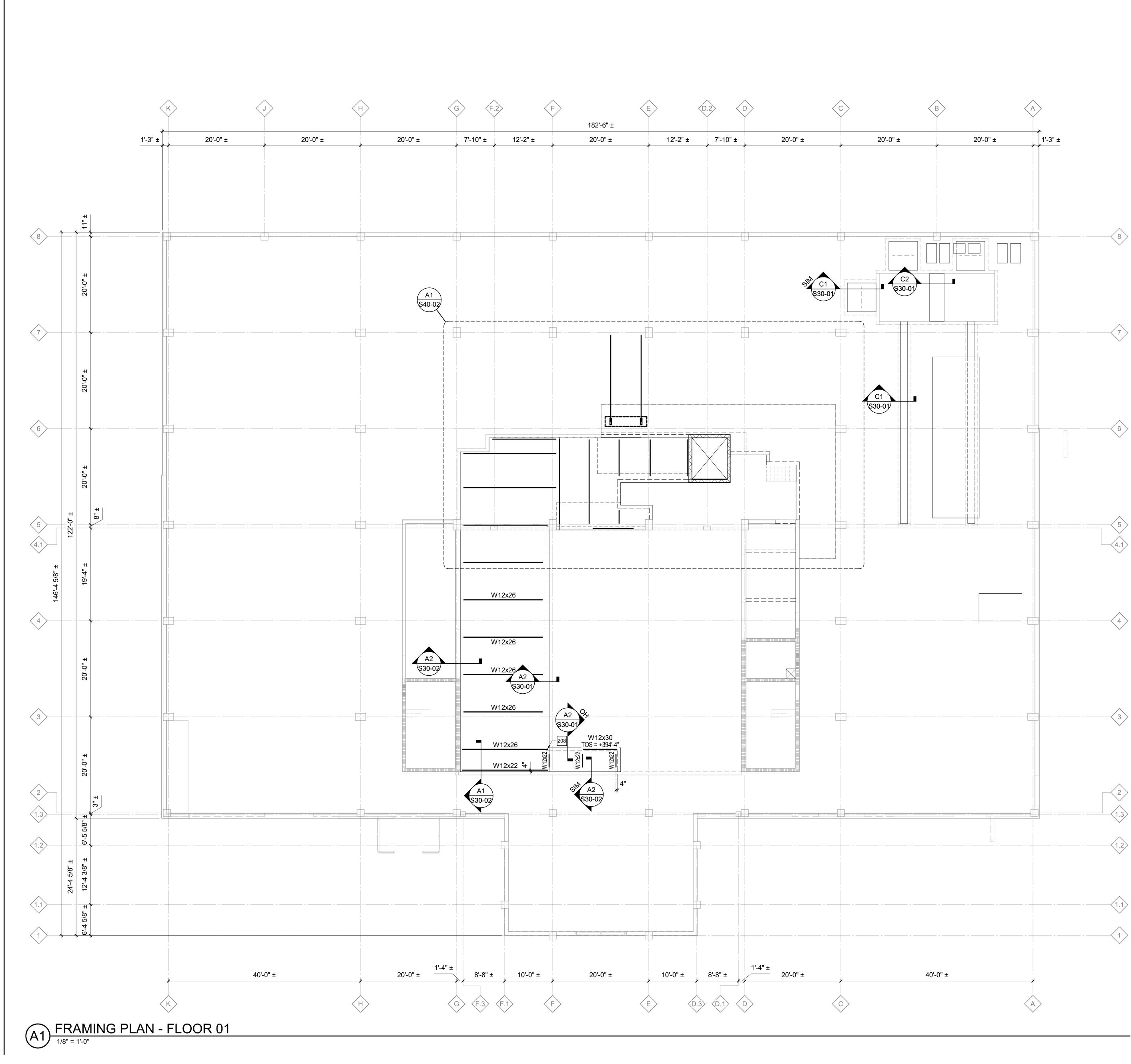
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- D. FLOOR SINKS AND DRAINS ARE NOT SHOWN ON PLAN. REFERENCE PME DRAWINGS FOR LOCATIONS.
- E. REFERENCE CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.
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KEY NOTES

101	2'-0" SQUARE SUMP PIT FOR ELEVATOR. REFERENCE
	TYPICAL DETAIL. COORDINATE LOCATION WITH
	PLUMBING DRAWINGS AND ELEVATOR MANUFACTURER.
109	8" CMU WALL REINFORCED WITH #5 VERTICAL AT
	40" OC, CENTERED IN WALL.
X02	EXISTING CONCRETE COLUMN.

- EXISTING CONCRETE CAISSON. X03
- EXISTING CONCRETE GRADE BEAM. X04



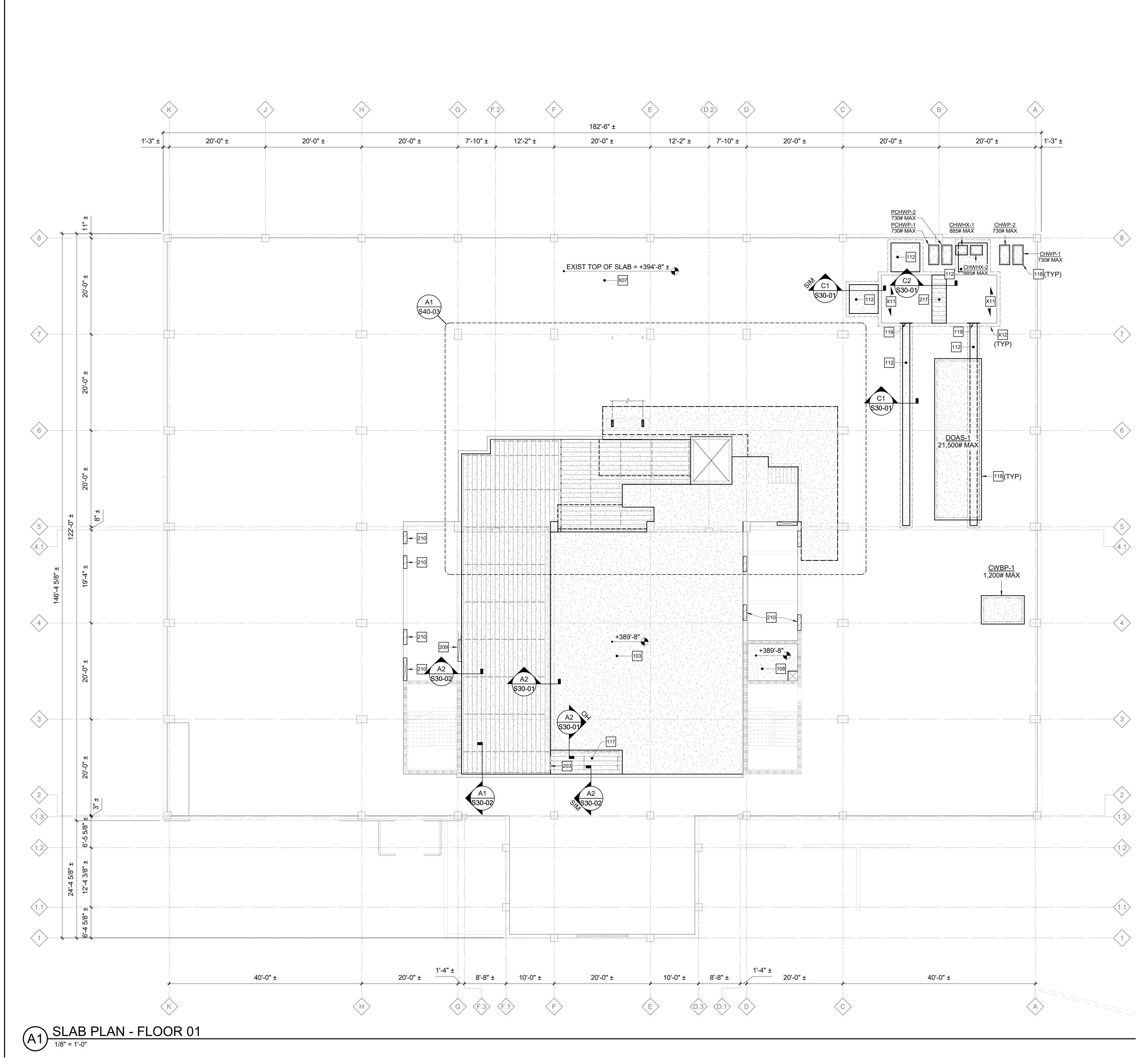


- A. REFERENCE FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- B. TOP OF FINISHED FLOOR ELEVATION MUST BE AS NOTED ON SLAB PLANS.
- C. STEEL FLOOR FRAMING MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 8'-0" ON-CENTER).
- D. STEEL ROOF FRAMING SUPPORTING 1 1/2" STEEL ROOF DECK MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 5'-0" ON-CENTER).
- E. CONCRETE ON ELEVATED METAL DECKS MUST BE POURED TO THE THICKNESS INDICATED.
- F. AT STEEL ROOF FRAMING, BOTTOM OF DECK ELEVATIONS ARE SHOWN ON PLAN. INTERMEDIATE ELEVATIONS MUST BE STRAIGHT LINES BETWEEN GIVEN ELEVATIONS. INTERPOLATE AS REQUIRED FOR INTERMEDIATE BEARING ELEVATIONS, UNLESS OTHERWISE NOTED.
- G. COORDINATE AND VERIFY ALL MEMBER LOCATIONS. DIMENSIONS, WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR ALL MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED. INCLUDE THIS INFORMATION ON THE JOIST AND STRUCTURAL STEEL SHOP DRAWINGS.
- H. EXTENTS OF SLAB/JOIST DEMO AND NEW MEMBER FRAMING LENGTHS ARE APPROXIMATE. EXISTING FRAMING CONDITIONS AND REQUIRED MEASUREMENTS MUST BE FIELD VERIFIED PRIOR TO DEMOLITION AND FABRICATION. DESIGN INTENT IS FOR NEW OPENING AND SLAB CONSTRUCTION TO BE LOCATED BETWEEN EXISTING CONCRETE JOISTS, ADJUST DIMENSIONS AND LOCATION OF SLAB DEMO AS NEEDED. NOTIFY ENGINEER IF AS-BUILT CONDITIONS ARE INCONSISTENT WITH INFORMATION INDICATED ON PLAN.
- DIMENSIONS TO CHANNELS ARE FROM FLAT FACE OF CHANNEL.

KEY NOTES

EXTEND LONG W12 BEAM TO BEAR ON CMU WALL. FRAME 208 SHORT W12 BEAM INTO LONG W12 BEAM.





SLAB/DECK PLAN NOTES

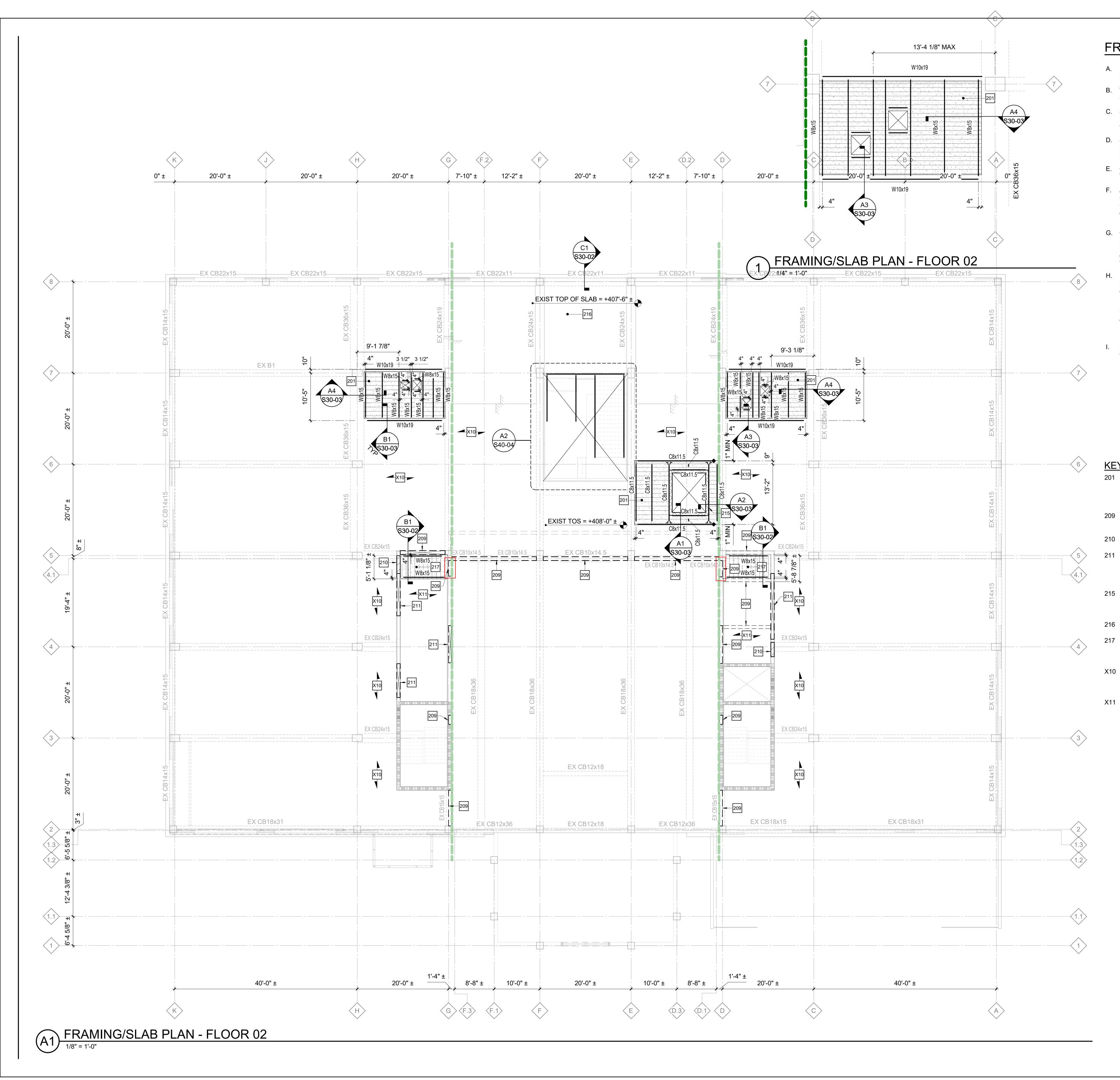
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KEY NOTES

- 4" TOPPING SLAB OVER EXISTING SLAB. REINFORCE SLAB 103 WITH 6x6-W2.1xW2.1 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT. APPLY SIKA BONDBREAKER ID OR APPROVED EQUIVALENT BETWEEN EXISTING SLAB AND TOPPING SLAB. 108 REMOVE EXISTING CONCRETE SLAB AND REPLACE WITH 12" CONCRETE SLAB-ON-GRADE. REINFORCE SLAB WITH #4 AT 12" ON-CENTER EACH WAY, TOP AND BOTTOM. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT. 112 INFILL EXISTING PIT WITH COMPACTED #57 STONE. PLACE NEW 4" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND COMPACTED #57 STONE. REINFORCE SLAB WITH 6x6-W2.1xW2.1 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN **REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR** SPACERS DURING CONCRETE PLACEMENT, DOWEL TO ADJACENT EXISTING CONCRETE SLAB, REFER TO TYPICAL SLAB DOWEL DETAIL. 4" TOPPING SLAB EXTENDS INTO STORAGE AREA BELOW 117 ELEVATED SLAB. REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT EXTENTS. CONCRETE HOUSEKEEPING PAD AT MECHANICAL 118 EQUIPMENT, REFER TO HOUSEKEEPING PAD AT EXISTING SLAB ON GRADE DETAILS. COORDINATE EXACT SIZE AND LOCATION WITH MECHANICAL DRAWINGS. GALVANIZED CLOSURE PL1/2x24 FOR FULL DEPTH OF 119 OPENING BETWEEN EXISTING PIT AND TRENCH. FASTEN TO EXISTING CONCRETE WALLS WITH 5/8"⌀ SCREW ANCHORS WITH 5" EMBEDMENT AT 12" ON-CENTER EACH SIDE OF OPENING (21" GAGE). CHANGE DECK DIRECTION AT BEAM CENTER LINE. 203 209 NEW OPENING IN EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE. REFER TO TYPICAL STEEL LINTEL BEARING ON EXISTING MASONRY DETAILS. 210 INFILL EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE WITH BRICK TO MATCH EXISTING. 2 1/2" NORMAL WEIGHT CONCRETE SLAB ON 1 1/2" FORM 217 DECK, (4" TOTAL) REINFORCED WITH 6x6-W2.9xW2.9 WELDED WIRE REINFORCING LOCATED 1" CLEAR BELOW
 - TOP OF SLAB. EXISTING 4" CONCRETE SLAB-ON-GRADE.
- X07 EXISTING 6" CONCRETE SLAB, SPAN DIRECTION OF
- PRIMARY REINFORCEMENT INDICATED ON PLAN. X12
 - EXISTING 8" CONCRETE WALL.

X11





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FRAMING PLAN NOTES

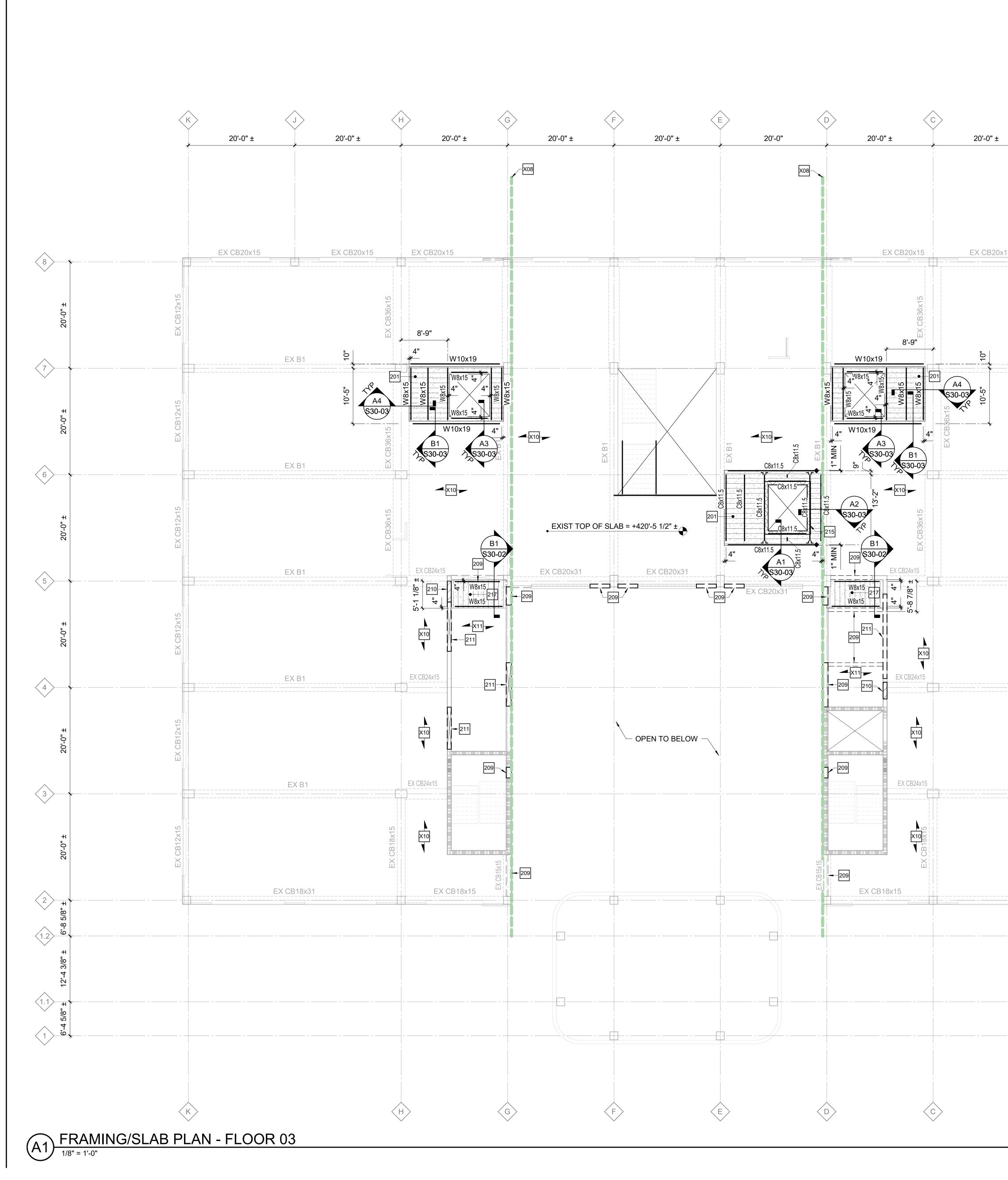
- A. REFERENCE FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- B. TOP OF FINISHED FLOOR ELEVATION MUST BE AS NOTED ON SLAB PLANS.
- C. STEEL FLOOR FRAMING MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 8'-0" ON-CENTER).
- D. STEEL ROOF FRAMING SUPPORTING 1 1/2" STEEL ROOF DECK MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 5'-0" ON-CENTER).
- E. CONCRETE ON ELEVATED METAL DECKS MUST BE POURED TO THE THICKNESS INDICATED.
- F. AT STEEL ROOF FRAMING, BOTTOM OF DECK ELEVATIONS ARE SHOWN ON PLAN. INTERMEDIATE ELEVATIONS MUST BE STRAIGHT LINES BETWEEN GIVEN ELEVATIONS. INTERPOLATE AS REQUIRED FOR INTERMEDIATE BEARING ELEVATIONS, UNLESS OTHERWISE NOTED.
- G. COORDINATE AND VERIFY ALL MEMBER LOCATIONS, DIMENSIONS, WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR ALL MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED. INCLUDE THIS INFORMATION ON THE JOIST AND STRUCTURAL STEEL SHOP DRAWINGS.
- H. EXTENTS OF SLAB/JOIST DEMO AND NEW MEMBER FRAMING LENGTHS ARE APPROXIMATE. EXISTING FRAMING CONDITIONS AND REQUIRED MEASUREMENTS MUST BE FIELD VERIFIED PRIOR TO DEMOLITION AND FABRICATION. DESIGN INTENT IS FOR NEW OPENING AND SLAB CONSTRUCTION TO BE LOCATED BETWEEN EXISTING CONCRETE JOISTS, ADJUST DIMENSIONS AND LOCATION OF SLAB DEMO AS NEEDED. NOTIFY ENGINEER IF AS-BUILT CONDITIONS ARE INCONSISTENT WITH INFORMATION INDICATED ON PLAN.
- I. DIMENSIONS TO CHANNELS ARE FROM FLAT FACE OF CHANNEL.

KEY NOTES

201	REMOVE EXISTING CONCRETE SLAB/JOISTS AND REPLACE WITH 2 1/2" NORMAL WEIGHT CONCRETE SLAB ON 1 1/2" FORM FLOOR DECK, (4" TOTAL) REINFORCED WITH 6x6-W2.9xW2.9 WELDED WIRE REINFORCING LOCATED 1" CLEAR BELOW TOP OF SLAB.
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210	INFILL EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE WITH BRICK TO MATCH EXISTING.
211	WIDEN OPENING IN EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE AND REMOVE EXISTING STEEL LINTEL. REFER TO TYPICAL STEEL LINTEL ON EXISTING MASONRY DETAILS. REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATION OF LINTELS.
215	LOCATE BEAM DIRECTLY BELOW NEW STAIR STRINGERS/POSTS. REFER TO THE ARCHITECTURAL DRAWINGS FOR STRINGER/POST BEARING LOCATION. PROVIDE BEARING CONNECTION ONLY.
216	EXISTING NON-STRUCTURAL SLAB AND RIGID INSULATION TO BE REMOVED AND REPLACED IN KIND.
217	2 1/2" NORMAL WEIGHT CONCRETE SLAB ON 1 1/2" FORM DECK, (4" TOTAL) REINFORCED WITH 6x6-W2.9xW2.9 WELDED WIRE REINFORCING LOCATED 1" CLEAR BELOW TOP OF SLAB.
X10	EXISTING 3" CONCRETE SLAB AND 15" CONCRETE PAN

- EXISTING 3" CONCRETE SLAB AND 15" CONCRETE PAN JOISTS, SPAN DIRECTION OF PAN JOISTS INDICATED ON PLAN. FIELD VERIFY JOIST SPACING AND DEPTH PRIOR TO DEMO.
- EXISTING 6" CONCRETE SLAB, SPAN DIRECTION OF PRIMARY REINFORCEMENT INDICATED ON PLAN.





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KEY NOTES

X11

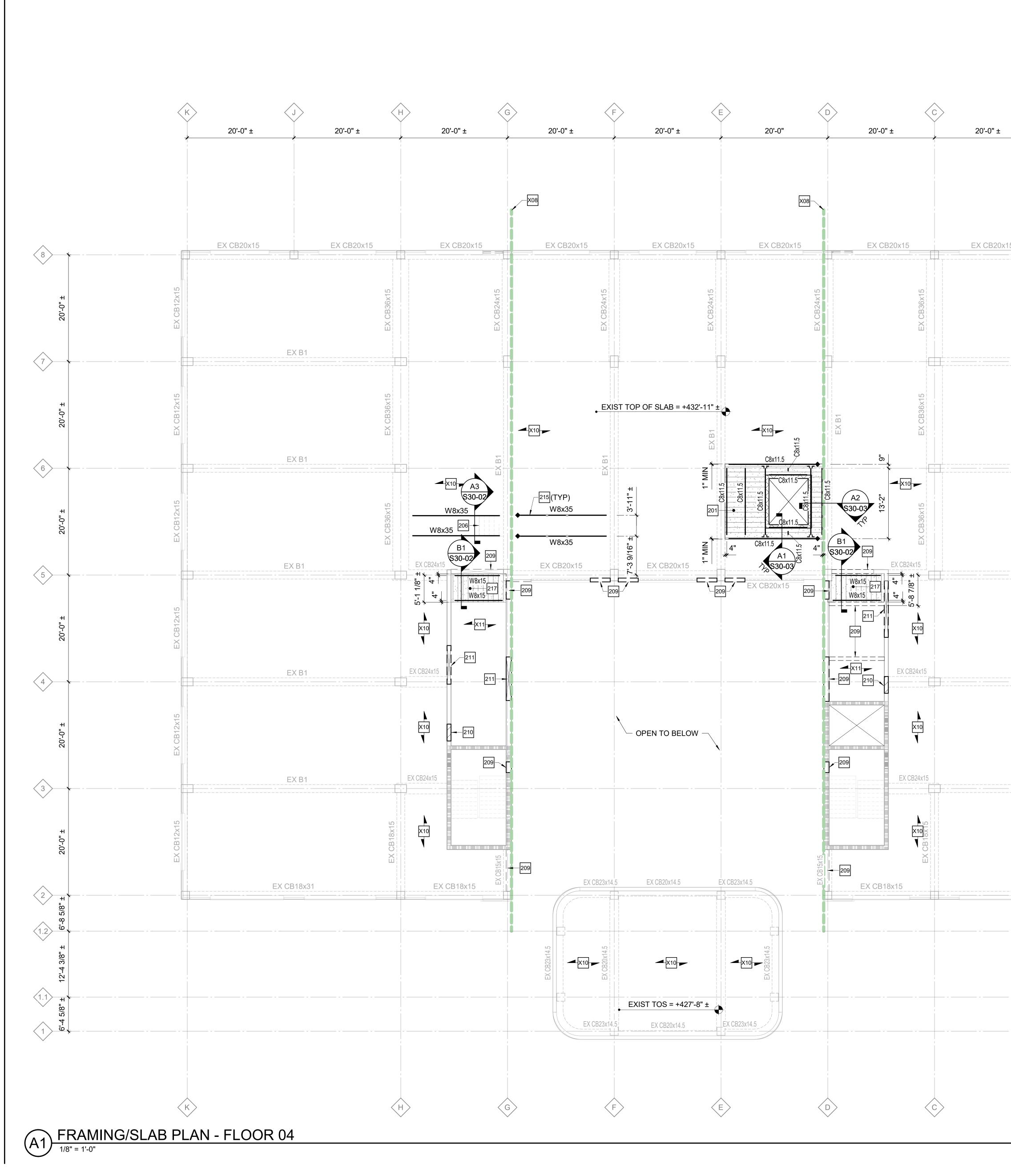
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PRIMARY REINFORCEMENT INDICATED ON PLAN.

B	Â	
20'-0" ±	/	
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	EX CB12x15	
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	EX CB12x15	
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	12x15	
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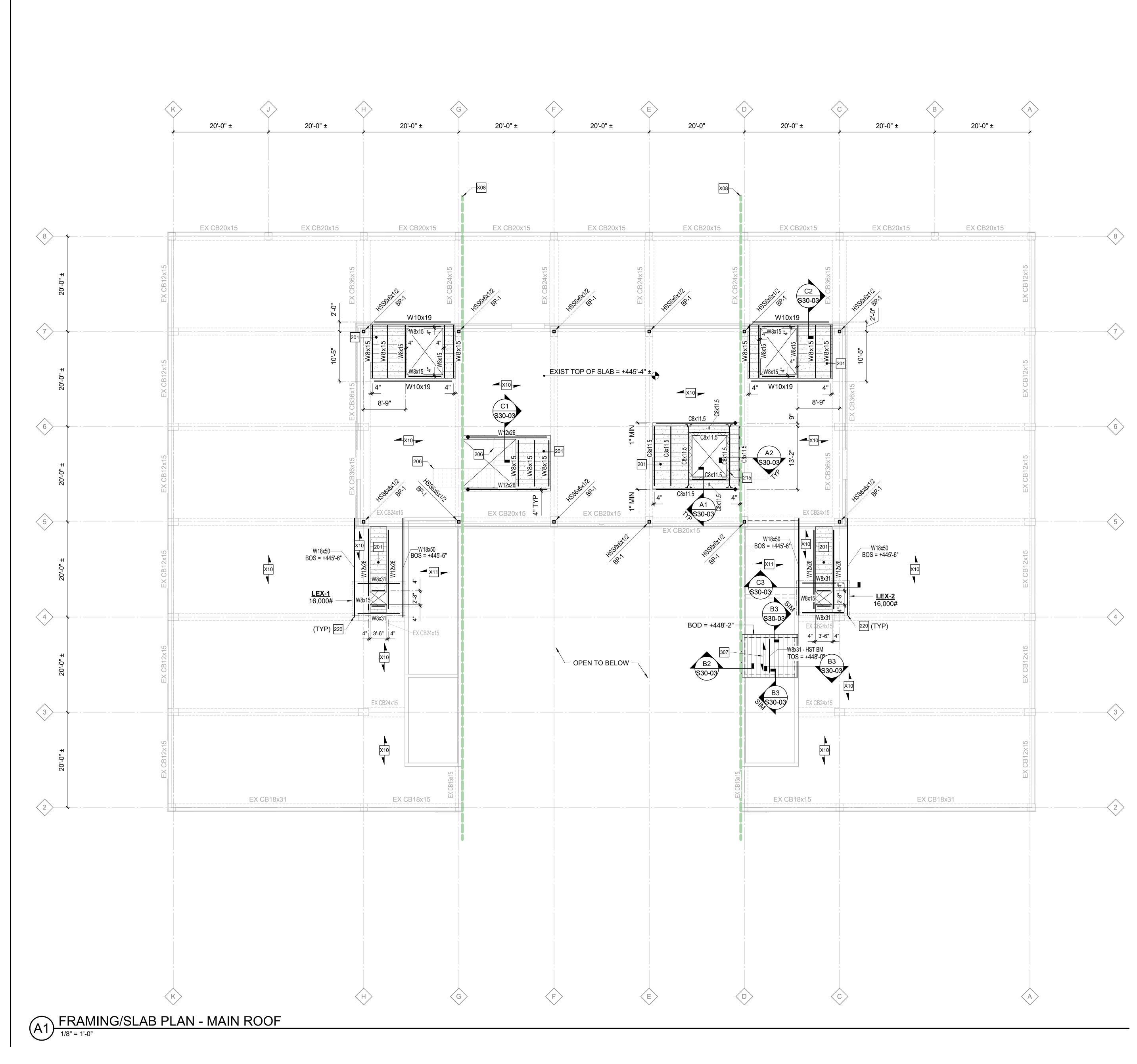
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B	20'-0" ±		
15	EX CB20x15	2x15	
EX B1		EX CB12x15	
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FRAMING PLAN NOTES

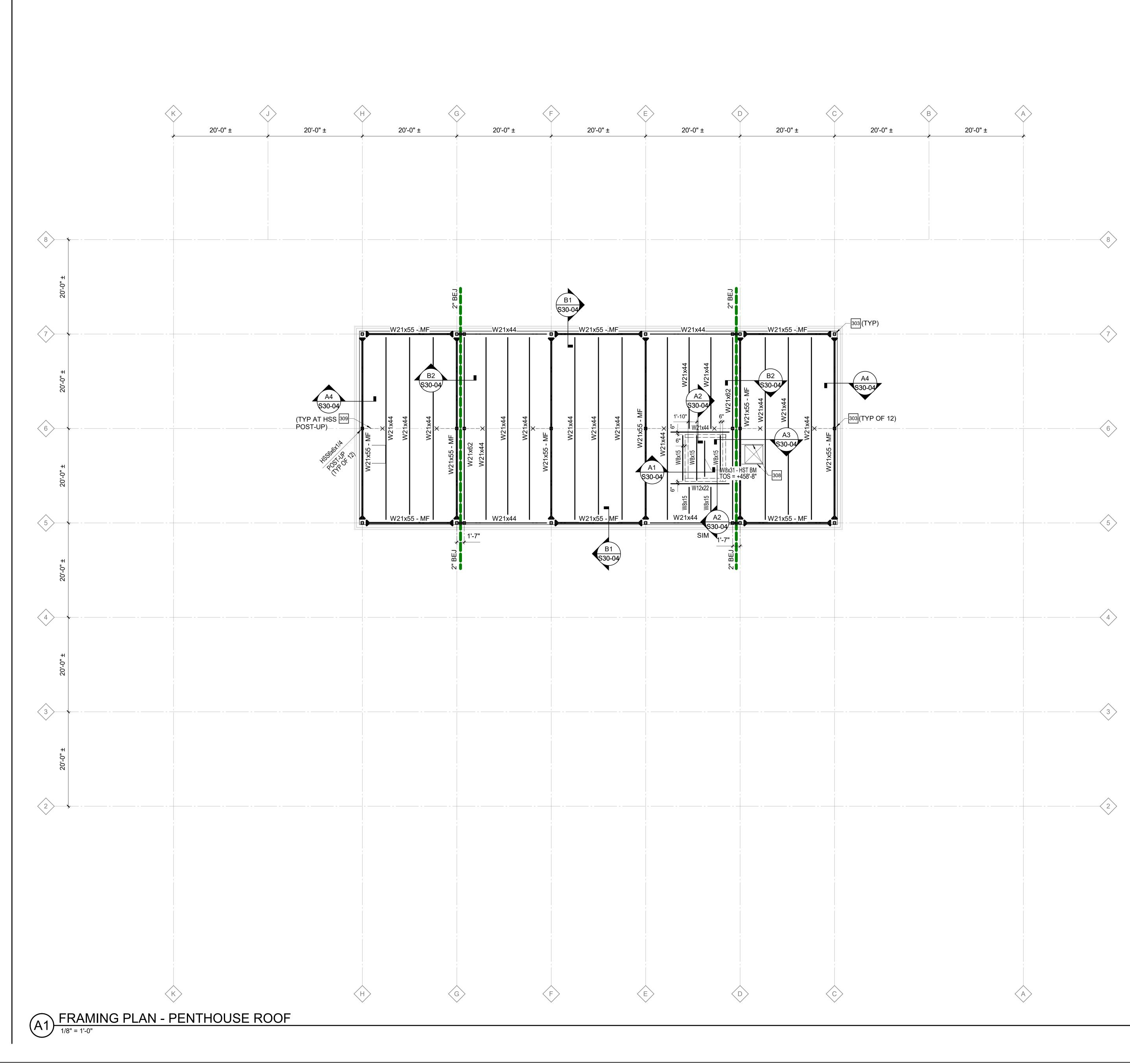
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220	BEARING PL2x12x12 AT EACH END OF EXHAUST FAN SUPPORT BEAM.
307	3" x 20 GAGE ROOF DECK. REFERENCE STEEL DECK NOTES.
X08	EXISTING 1" +/- BUILDING EXPANSION JOINT.
X10	EXISTING 3" CONCRETE SLAB AND 15" CONCRETE PAN JOISTS, SPAN DIRECTION OF PAN JOISTS INDICATED ON PLAN. FIELD VERIFY JOIST SPACING AND DEPTH PRIOR TO DEMO.

X11 EXISTING 6" CONCRETE SLAB, SPAN DIRECTION OF PRIMARY REINFORCEMENT INDICATED ON PLAN.





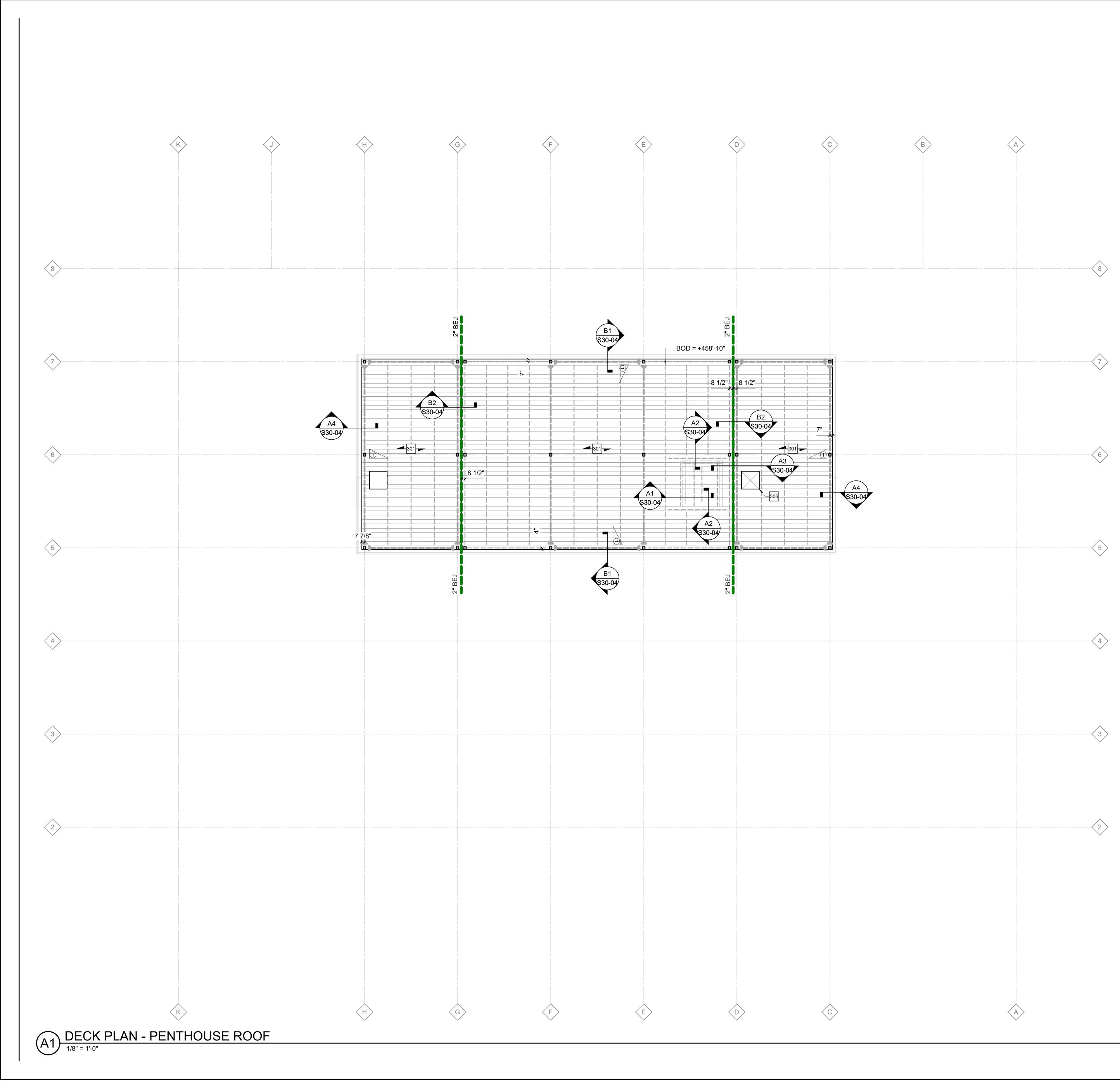
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KEY NOTES

303	ALTERNATE #4 - EXTEND TOP OF COLUMN 28" ABOVE BOTTOM OF DECK FOR FUTURE PHOTOVOLTAIC PANEL SYSTEM SUPPORT. COLUMN MUST BE HOT-DIP GALVANIZED.
308	L5x5x5/16 AT ROOF HATCH.
000	

REFER TO TYPICAL BEAM BOTTOM FLANGE BRACE DETAILS 309





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SLAB/DECK PLAN NOTES

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- B. ACTUAL FINISHED FLOOR ELEVATIONS ARE SPECIFIED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIAL.
- C. REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT LIMITS OF SLAB DEPRESSIONS AND OMITTED SLABS.
- D. FLOOR SINKS AND DRAINS ARE NOT SHOWN ON PLAN. REFERENCE PME DRAWINGS FOR LOCATIONS.
- E. REFERENCE CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.
- F. SLAB-ON-GRADE JOINTS MUST BE SAWED JOINTS OR KEYED CONSTRUCTION JOINTS, UNLESS OTHERWISE NOTED. CONTRACTOR MUST COORDINATE ALL SLAB JOINTS WITH JOINTS IN BONDED FLOOR FINISHES. REFERENCE ARCHITECTURAL DRAWINGS FOR FLOOR FINISH JOINT LOCATIONS.
- G. PLACE (1) #4 x 3'-0" IN MIDDEPTH OF SLAB AT RE-ENTRANT CORNERS WHERE A SLAB JOINT DOES NOT OCCUR.
- H. EXISTING SLAB-ON-GRADE IS 4" CONCRETE, UNLESS OTHERWISE NOTED.

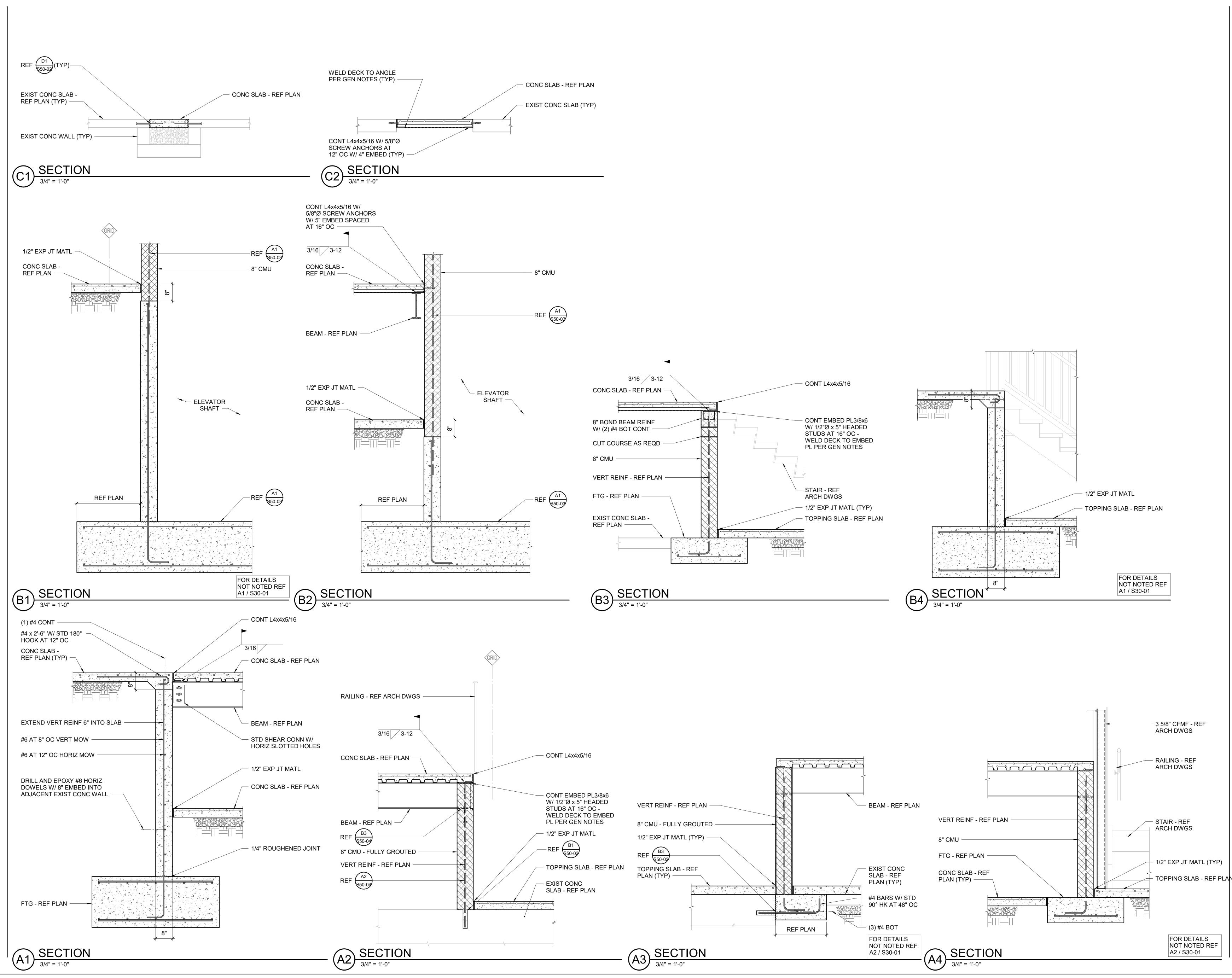
KEY NOTES

301	1 1/2" x 22 GAGE ROOF DECK. REFERENCE STEEL DECK
	NOTES.

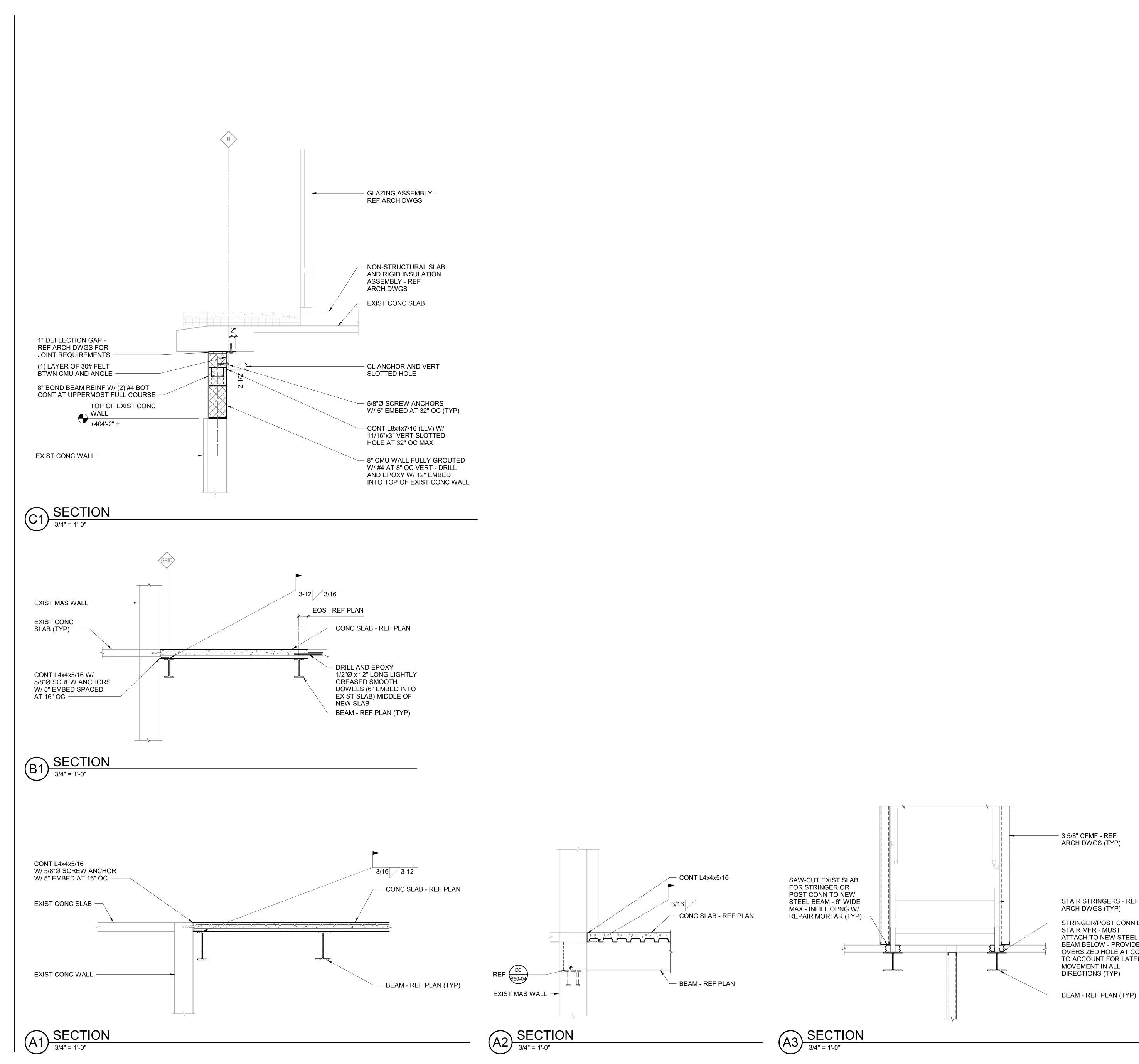
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NOTES. ROOF HATCH, REFER TO ARCHITECTURAL DRAWINGS FOR EXACT SIZE AND LOCATION.





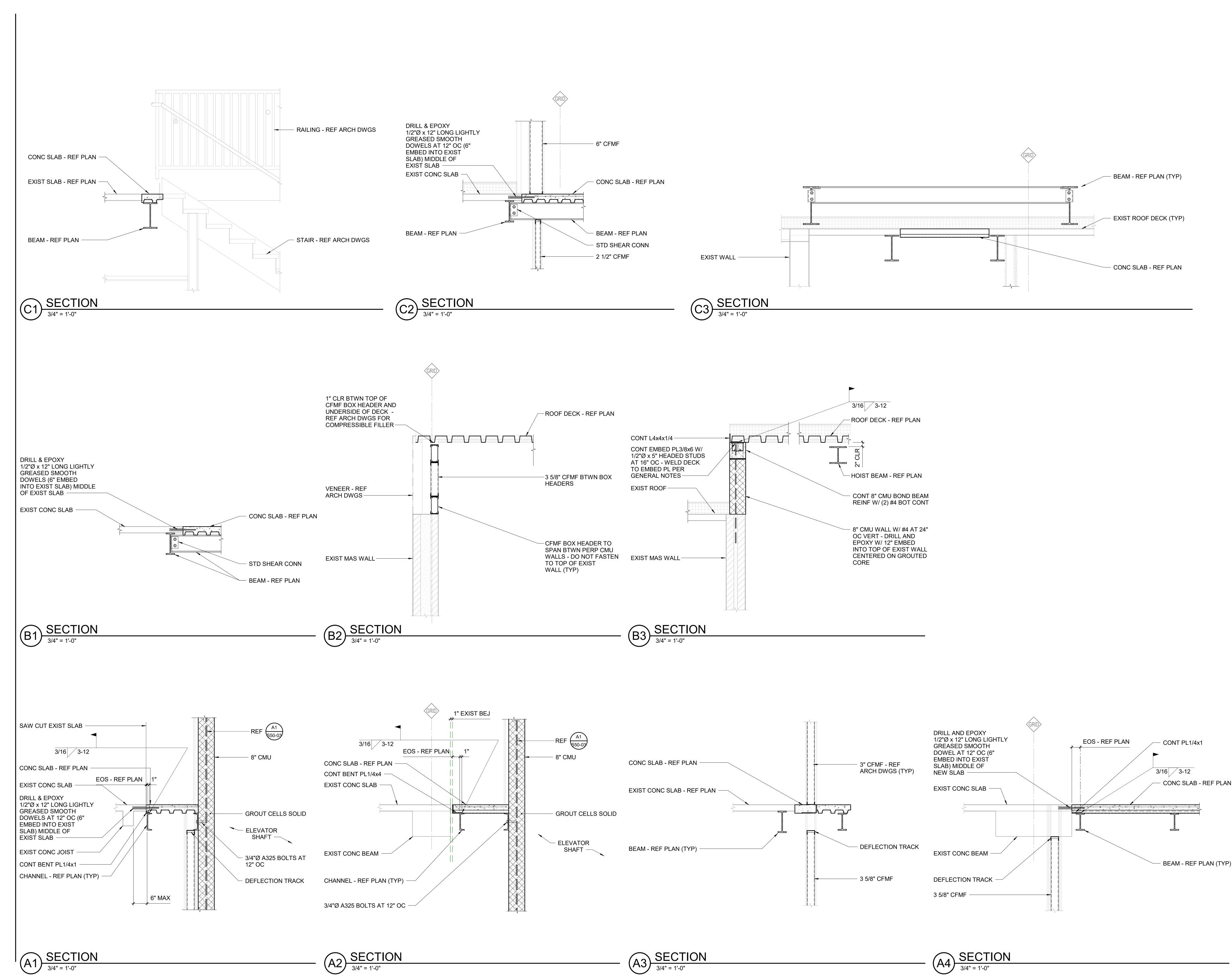




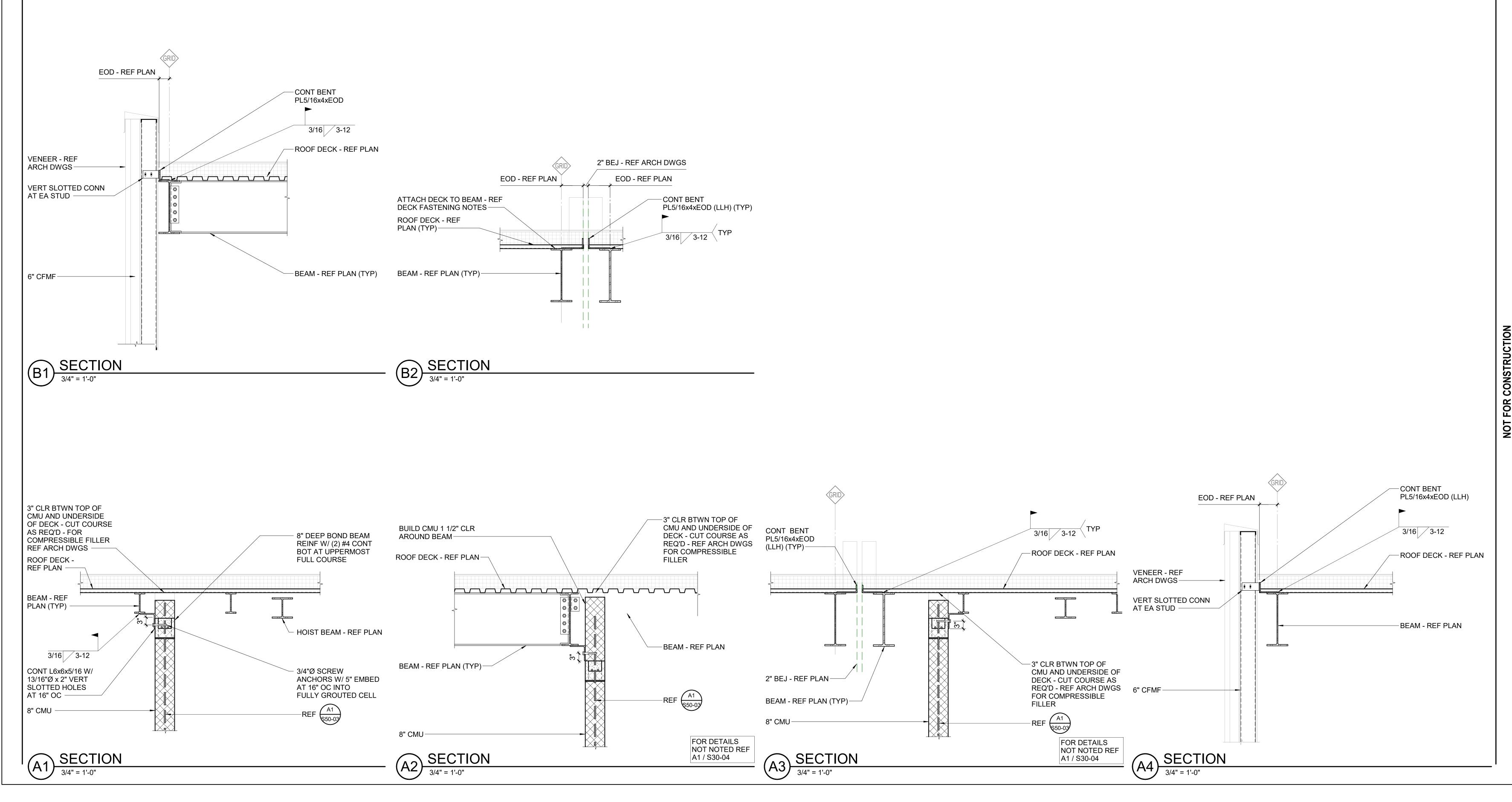
- STAIR STRINGERS - REF ARCH DWGS (TYP)

STRINGER/POST CONN BY STAIR MFR - MUST ATTACH TO NEW STEEL BEAM BELOW - PROVIDE OVERSIZED HOLE AT CONN TO ACCOUNT FOR LATERAL

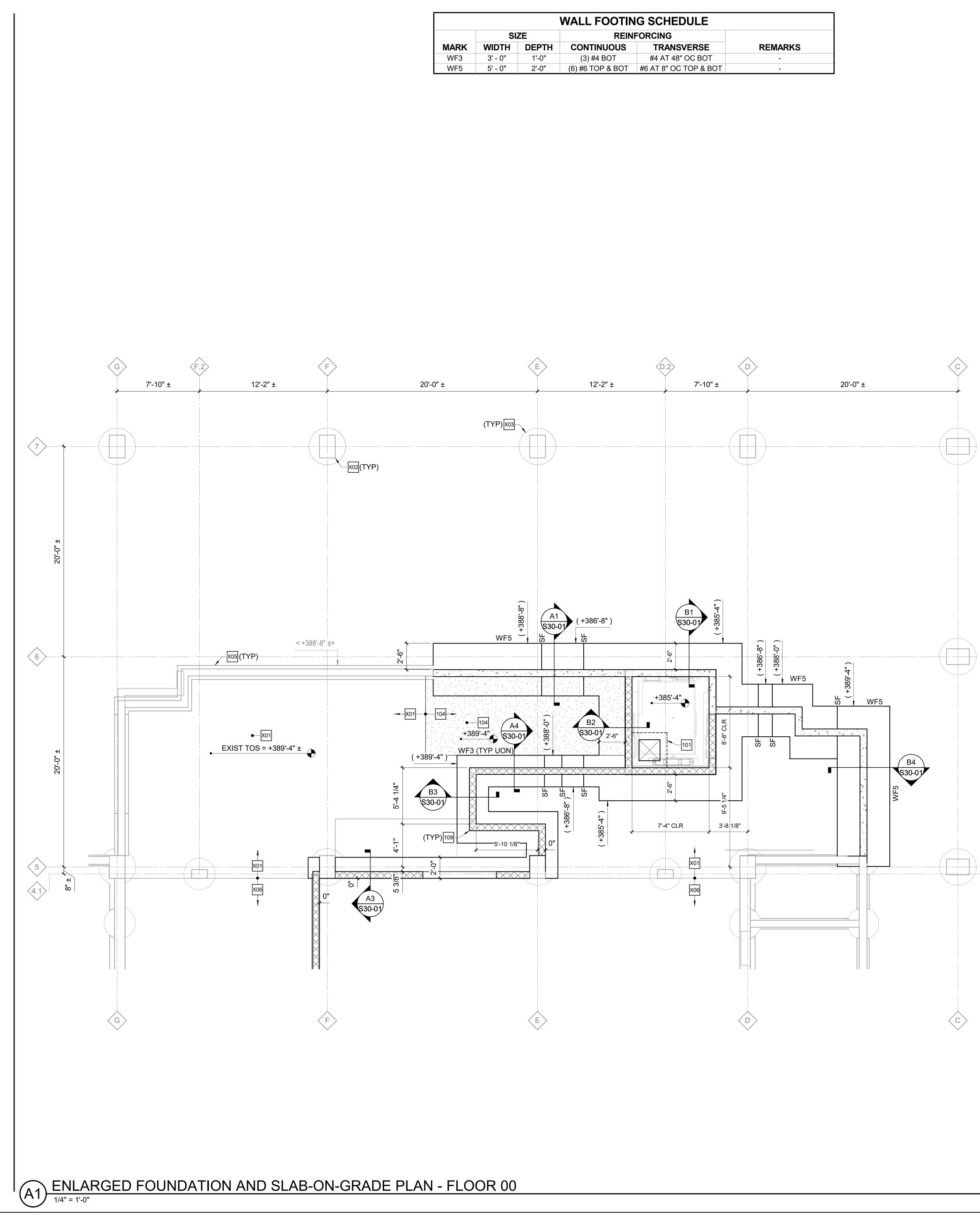












WALL FOOTING SCHEDULE				
SI	ZE	REINFORCING		
WIDTH	DEPTH	CONTINUOUS	TRANSVERSE	REMARKS
3' - 0"	1'-0"	(3) #4 BOT	#4 AT 48" OC BOT	-
5' - 0"	2'-0"	(6) #6 TOP & BOT	#6 AT 8" OC TOP & BOT	-

FOUNDATION PLAN NOTES

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- B. ACTUAL FINISHED FLOOR ELEVATIONS ARE SPECIFIED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIAL.
- C. TOP OF ALL FOOTINGS ARE INDICATED ON PLAN.
- D. NOT ALL UTILITY LOCATIONS ARE SHOWN ON PLAN. THE CONTRACTOR MUST COORDINATE THE LOCATIONS, SIZES, AND INVERTS OF UTILITIES. AT LOCATIONS WHERE UTILITIES PASS BELOW THE TOP OF FOOTING ELEVATION, STEP THE TOP OF FOOTING DOWN ON EACH SIDE PER THE "TYPICAL STEPPED FOOTING DETAIL" AND SLEEVE THE UTILITY THROUGH THE FOUNDATION WALL. THE CONTRACTOR MAY, AT HIS/HER OPTION, SLEEVE THE UTILITY THROUGH THE FOUNDATION PER THE "TYPICAL PIPE SLEEVE AT WALL FOOTING DETAILS."
- E. UNLESS OTHERWISE INDICATED, EXTEND WALL FOOTINGS A MINIMUM OF 6 INCHES BEYOND ENDS OF WALLS.
- F. SITE WALLS ARE NOT SHOWN ON PLAN. CONTRACTOR MUST COORDINATE CIVIL AND LANDSCAPE DRAWINGS FOR SITE WALL INFORMATION.
- G. DIMENSIONS SHOWN ON FOUNDATION PLAN ARE TO COLUMN GRIDLINES AND OUTSIDE FACE OF FOUNDATION WALLS, UNLESS OTHERWISE NOTED.

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- D. FLOOR SINKS AND DRAINS ARE NOT SHOWN ON PLAN. REFERENCE PME DRAWINGS FOR LOCATIONS.
- E. REFERENCE CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.
- F. SLAB-ON-GRADE JOINTS MUST BE SAWED JOINTS OR KEYED CONSTRUCTION JOINTS, UNLESS OTHERWISE NOTED. CONTRACTOR MUST COORDINATE ALL SLAB JOINTS WITH JOINTS IN BONDED FLOOR FINISHES. REFERENCE ARCHITECTURAL DRAWINGS FOR FLOOR FINISH JOINT LOCATIONS.
- G. PLACE (1) #4 x 3'-0" IN MIDDEPTH OF SLAB AT RE-ENTRANT CORNERS WHERE A SLAB JOINT DOES NOT OCCUR.
- H. EXISTING SLAB-ON-GRADE IS 4" CONCRETE, UNLESS OTHERWISE NOTED.

KEY NOTES

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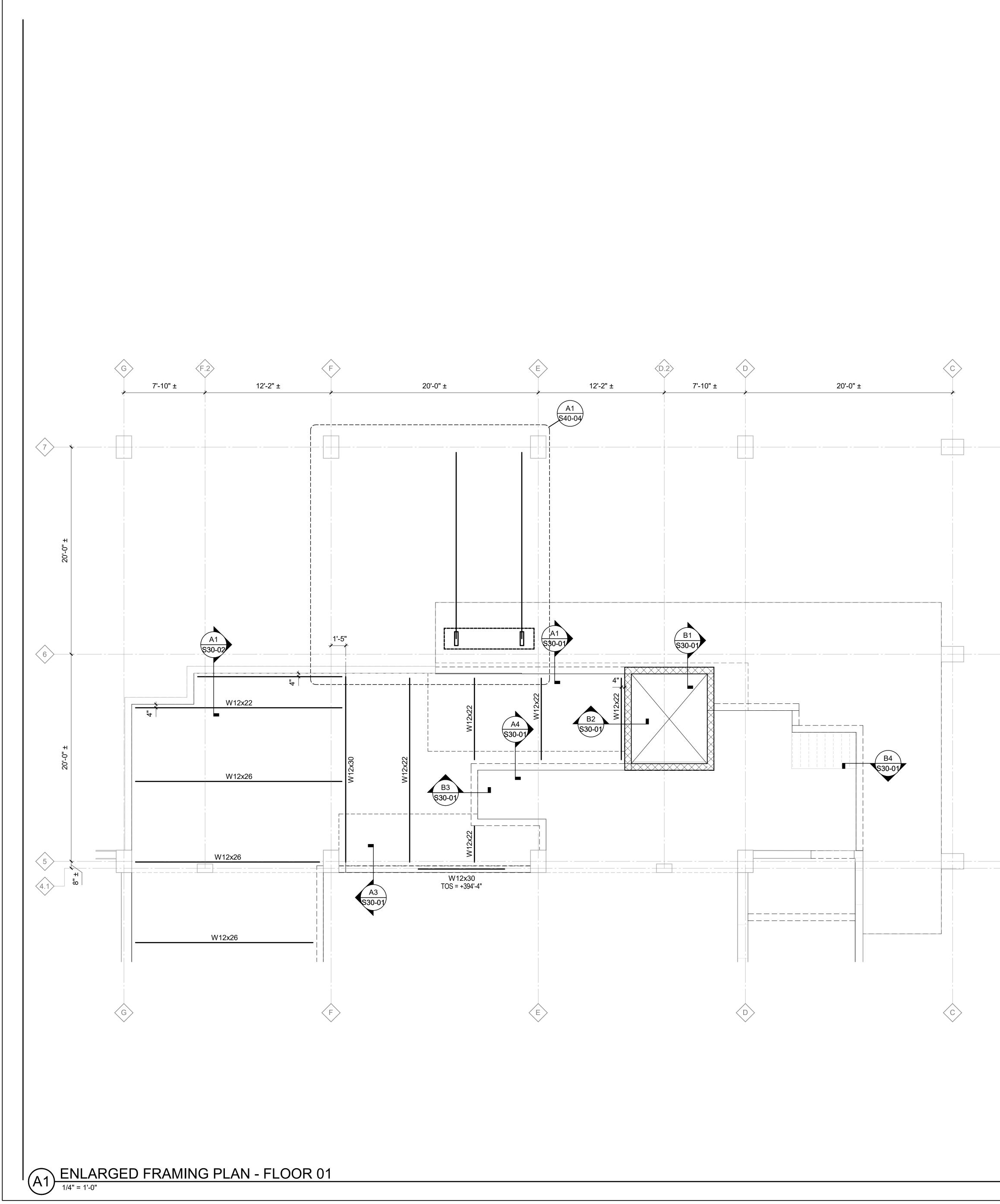
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101	2'-0" SQUARE SUMP PIT FOR ELEVATOR. REFERENCE TYPICAL DETAIL. COORDINATE LOCATION WITH PLUMBING DRAWINGS AND ELEVATOR MANUFACTURER.
104	SAW-CUT AND REPLACE EXISTING CONCRETE SLAB-ON-GRADE WITH 4" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND 4" DEPTH OF POROUS FILL UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH 6x6-W2.1xW2.1 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT. REFER TO TYPICAL SLAB DOWEL DETAIL. EXTENTS OF WORK AS REQUIRED FOR NEW CONCRETE WALL AND FOOTING WORK. COORDINATE TOP OF NEW SLAB ELEVATION WITH ADJACENT EXISTING SLAB. CONTRACTOR MUST VERIFY EXISTING SLAB THICKNESS.
109	8" CMU WALL REINFORCED WITH #5 VERTICAL AT 40" OC, CENTERED IN WALL.
X01	EXISTING 5" CONCRETE SLAB-ON-GRADE.
X02	EXISTING CONCRETE COLUMN.
X03	EXISTING CONCRETE CAISSON.

EXISTING CONCRETE WALL FOOTING. X05 X06 EXISTING 54" CONCRETE SLAB-ON-GRADE.





- A. REFERENCE FOUNDATION PLAN AND ARCHITECTURAL DRAWINGS FOR DIMENSIONS NOT SHOWN.
- B. TOP OF FINISHED FLOOR ELEVATION MUST BE AS NOTED ON SLAB PLANS.
- C. STEEL FLOOR FRAMING MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 8'-0" ON-CENTER).
- D. STEEL ROOF FRAMING SUPPORTING 1 1/2" STEEL ROOF DECK MUST BE EQUALLY SPACED BETWEEN POINTS OF KNOWN DIMENSIONS (NOT TO EXCEED 5'-0" ON-CENTER).
- E. CONCRETE ON ELEVATED METAL DECKS MUST BE POURED TO THE THICKNESS INDICATED.
- F. AT STEEL ROOF FRAMING, BOTTOM OF DECK ELEVATIONS ARE SHOWN ON PLAN. INTERMEDIATE ELEVATIONS MUST BE STRAIGHT LINES BETWEEN GIVEN ELEVATIONS. INTERPOLATE AS REQUIRED FOR INTERMEDIATE BEARING ELEVATIONS, UNLESS OTHERWISE NOTED.
- G. COORDINATE AND VERIFY ALL MEMBER LOCATIONS, DIMENSIONS, WEIGHTS, OPENING SIZES, AND CURB DIMENSIONS FOR ALL MECHANICAL EQUIPMENT WITH THE ACTUAL EQUIPMENT FURNISHED. INCLUDE THIS INFORMATION ON THE JOIST AND STRUCTURAL STEEL SHOP DRAWINGS.
- H. EXTENTS OF SLAB/JOIST DEMO AND NEW MEMBER FRAMING LENGTHS ARE APPROXIMATE. EXISTING FRAMING CONDITIONS AND REQUIRED MEASUREMENTS MUST BE FIELD VERIFIED PRIOR TO DEMOLITION AND FABRICATION. DESIGN INTENT IS FOR NEW OPENING AND SLAB CONSTRUCTION TO BE LOCATED BETWEEN EXISTING CONCRETE JOISTS, ADJUST DIMENSIONS AND LOCATION OF SLAB DEMO AS NEEDED. NOTIFY ENGINEER IF AS-BUILT CONDITIONS ARE INCONSISTENT WITH INFORMATION INDICATED ON PLAN.
- I. DIMENSIONS TO CHANNELS ARE FROM FLAT FACE OF CHANNEL.

<u>KEY NOTES</u>

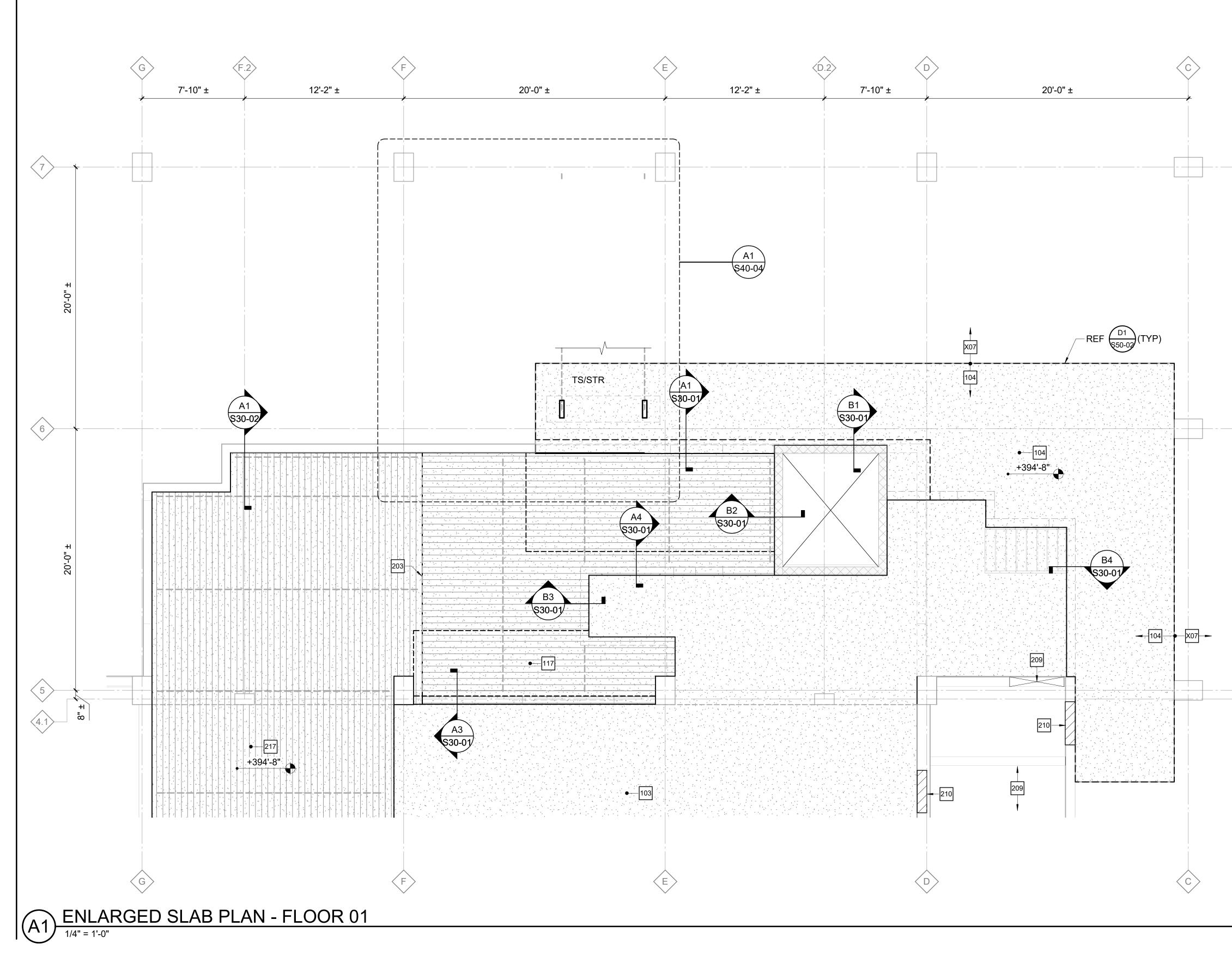
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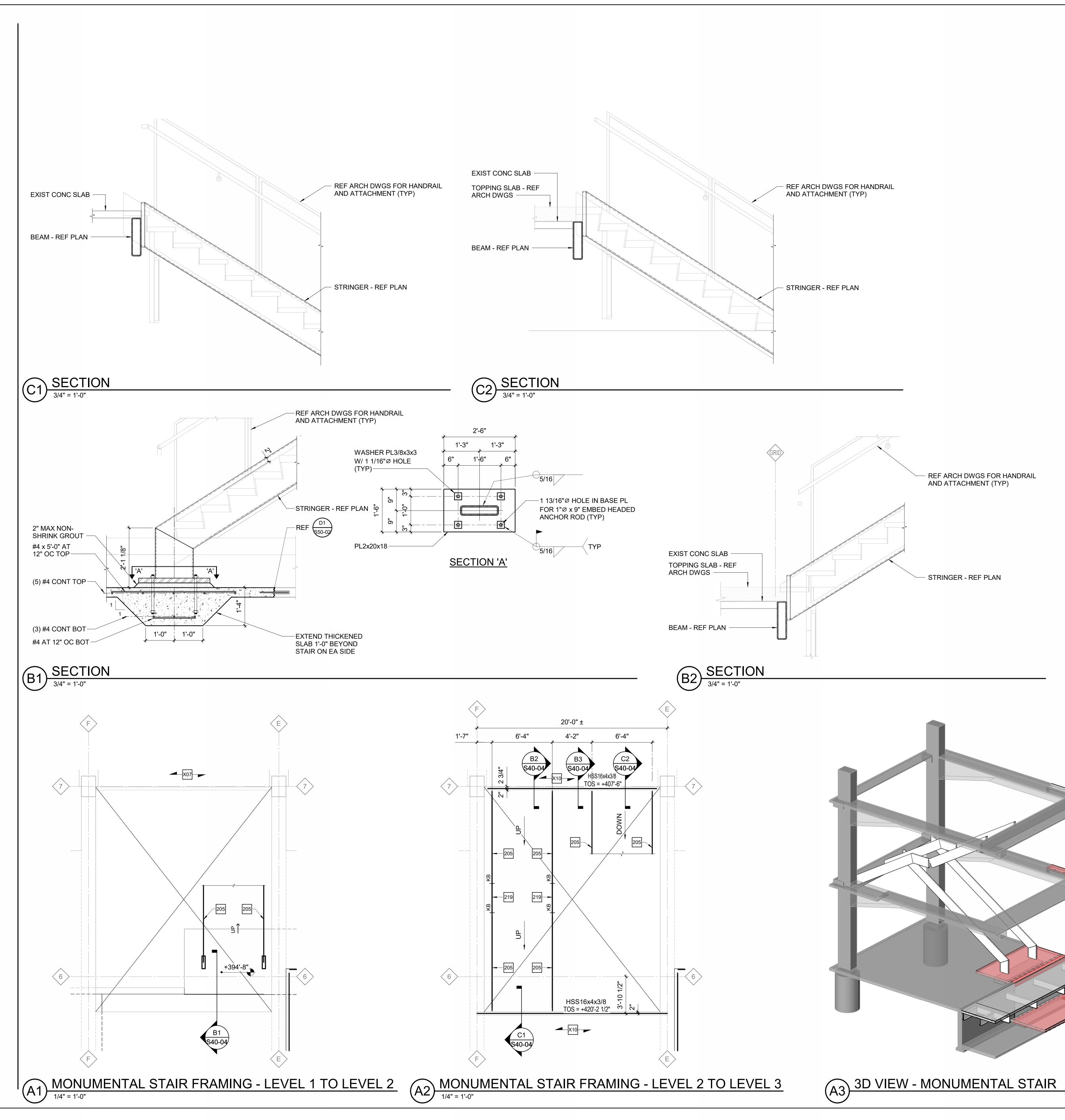
SLAB/DECK PLAN NOTES

- A. REFERENCE ARCHITECTURAL DRAWINGS FOR DIMENSIONS TO NONBEARING WALLS, WALL CONTROL JOINTS AND OPENINGS.
- B. ACTUAL FINISHED FLOOR ELEVATIONS ARE SPECIFIED ON PLAN. REFER TO ARCHITECTURAL DRAWINGS FOR FINISHED FLOOR MATERIAL.
- C. REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT LIMITS OF SLAB DEPRESSIONS AND OMITTED SLABS.
- D. FLOOR SINKS AND DRAINS ARE NOT SHOWN ON PLAN. REFERENCE PME DRAWINGS FOR LOCATIONS.
- E. REFERENCE CIVIL AND LANDSCAPE DRAWINGS FOR EXTERIOR CONCRETE SLABS AND PAVING.
- F. SLAB-ON-GRADE JOINTS MUST BE SAWED JOINTS OR KEYED CONSTRUCTION JOINTS, UNLESS OTHERWISE NOTED. CONTRACTOR MUST COORDINATE ALL SLAB JOINTS WITH JOINTS IN BONDED FLOOR FINISHES. REFERENCE ARCHITECTURAL DRAWINGS FOR FLOOR FINISH JOINT LOCATIONS.
- G. PLACE (1) #4 x 3'-0" IN MIDDEPTH OF SLAB AT RE-ENTRANT CORNERS WHERE A SLAB JOINT DOES NOT OCCUR.
- H. EXISTING SLAB-ON-GRADE IS 4" CONCRETE, UNLESS OTHERWISE NOTED.

KEY NOTES

103	4" TOPPING SLAB OVER EXISTING SLAB. REINFORCE SLAB WITH 6x6-W2.1xW2.1 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT. APPLY SIKA BONDBREAKER ID OR APPROVED EQUIVALENT BETWEEN EXISTING SLAB AND TOPPING SLAB.
104	SAW-CUT AND REPLACE EXISTING CONCRETE SLAB-ON-GRADE WITH 4" CONCRETE SLAB-ON-GRADE OVER VAPOR RETARDER AND 4" DEPTH OF POROUS FILL UNLESS OTHERWISE INDICATED. REINFORCE SLAB WITH 6x6-W2.1xW2.1 WELDED WIRE REINFORCING PLACED 1 1/2" CLEAR BELOW TOP OF SLAB. MAINTAIN REINFORCEMENT IN POSITION ON BOLSTERS, CHAIRS OR SPACERS DURING CONCRETE PLACEMENT. REFER TO TYPICAL SLAB DOWEL DETAIL. EXTENTS OF WORK AS REQUIRED FOR NEW CONCRETE WALL AND FOOTING WORK. COORDINATE TOP OF NEW SLAB ELEVATION WITH ADJACENT EXISTING SLAB. CONTRACTOR MUST VERIFY EXISTING SLAB THICKNESS.
117	4" TOPPING SLAB EXTENDS INTO STORAGE AREA BELOW ELEVATED SLAB. REFERENCE ARCHITECTURAL DRAWINGS FOR EXACT EXTENTS.
203	CHANGE DECK DIRECTION AT BEAM CENTER LINE.
209	NEW OPENING IN EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE. REFER TO TYPICAL STEEL LINTEL BEARING ON EXISTING MASONRY DETAILS.
210	INFILL EXISTING MULTI-WYTHE BRICK MASONRY WALL ABOVE WITH BRICK TO MATCH EXISTING.
217	2 1/2" NORMAL WEIGHT CONCRETE SLAB ON 1 1/2" FORM DECK, (4" TOTAL) REINFORCED WITH 6x6-W2.9xW2.9 WELDED WIRE REINFORCING LOCATED 1" CLEAR BELOW TOP OF SLAB.
X07	EXISTING 4" CONCRETE SLAB-ON-GRADE.

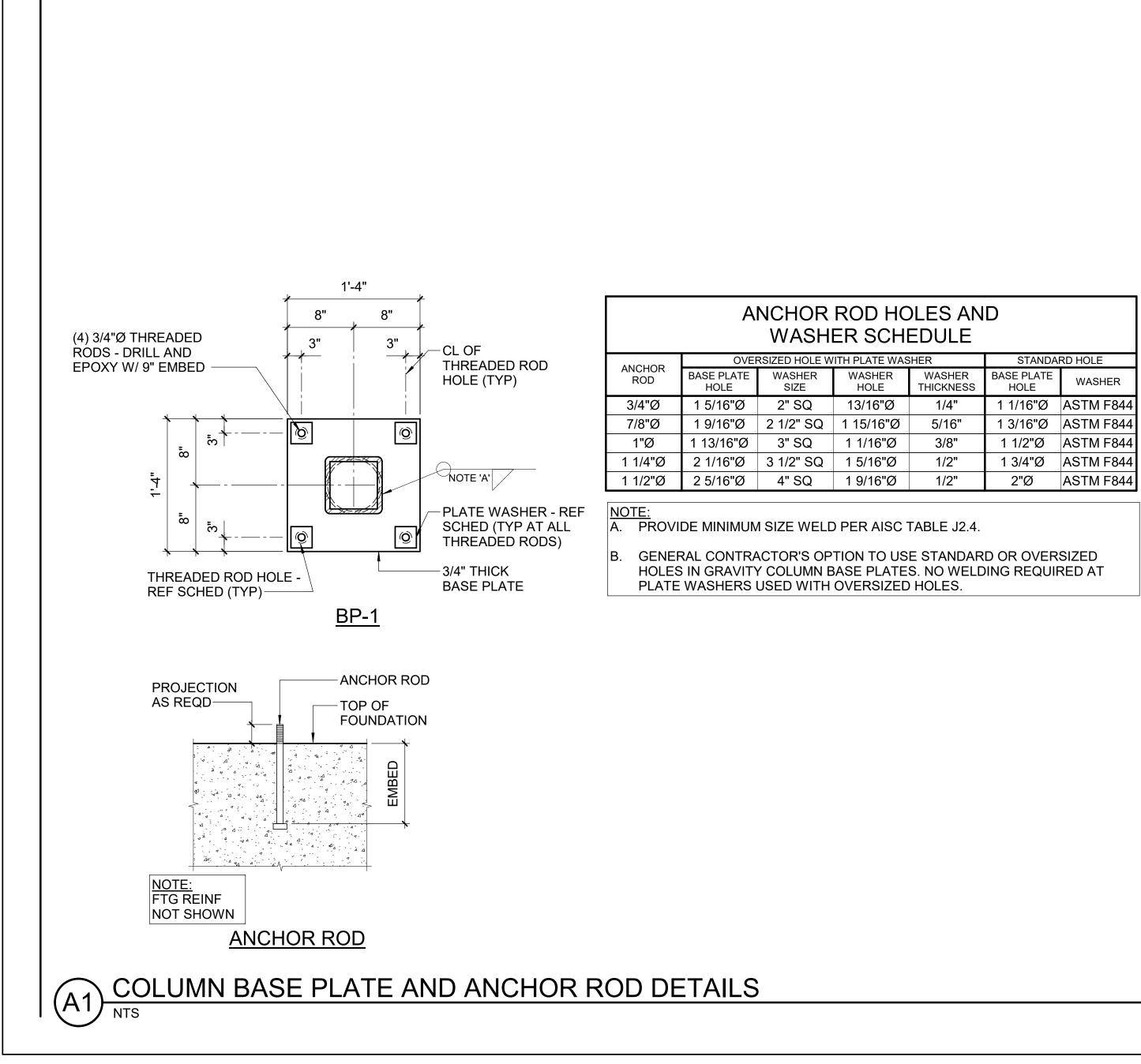




MONUMENTAL STAIR NOTES A. ALL MONUMENTAL STAIR FRAMING MUST BE ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS 3). B. SUPPLIER/CONTRACTOR MUST COORDINATE FIELD MEASUREMENTS PRIOR TO FABRICATION. C. ALL RAILINGS (STEEL OR OTHERWISE) AND THEIR CONNECTIONS MUST BE DESIGNED FOR A LATERAL LOAD OF 50 PLF APPLIED TO THE TOP OF THE RAIL OR A 200 POUND POINT LOAD IN ANY DIRECTION, WHICHEVER GOVERNS THE DESIGN. ALL RAILING SHOP DRAWINGS AND DESIGN CALCULATIONS MUST BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER LICENSED IN THE STATE OF NORTH CAROLINA. CALCULATIONS MUST BE SUBMITTED FOR REVIEW WITH THE RAILING SHOP DRAWINGS. KEY NOTES HSS16x4x3/8 STAIR STRINGER. 205 219 HSS16x4x3/8 AT STAIR LANDING. X07 EXISTING 4" CONCRETE SLAB-ON-GRADE. X10 EXISTING 3" CONCRETE SLAB AND 15" CONCRETE PAN JOISTS, SPAN DIRECTION OF PAN JOISTS INDICATED ON PLAN. FIELD VERIFY JOIST SPACING AND DEPTH PRIOR TO DEMO. - HANDRAIL - REF ARCH DWGS - REF ARCH DWGS FOR HANDRAIL AND ATTACHMENT (TYP) EXIST CONC SLAB -- STRINGER - REF PLAN BEAM - REF PLAN B3 <u>SECTION</u> 3/4" = 1'-0"







D HOLES AND SCHEDULE			
ATE WASHER STANDARD HOLE			RD HOLE
SHER DLE	WASHER THICKNESS	BASE PLATE HOLE	WASHER
16"Ø	1/4"	1 1/16"Ø	ASTM F84
/16"Ø	5/16"	1 3/16"Ø	ASTM F84
16"Ø	3/8"	1 1/2"Ø	ASTM F84
16"0	1/0"	1 2/1"0	ACTNEO

GENERAL CONTRACTOR'S OPTION TO USE STANDARD OR OVERSIZED HOLES IN GRAVITY COLUMN BASE PLATES. NO WELDING REQUIRED AT

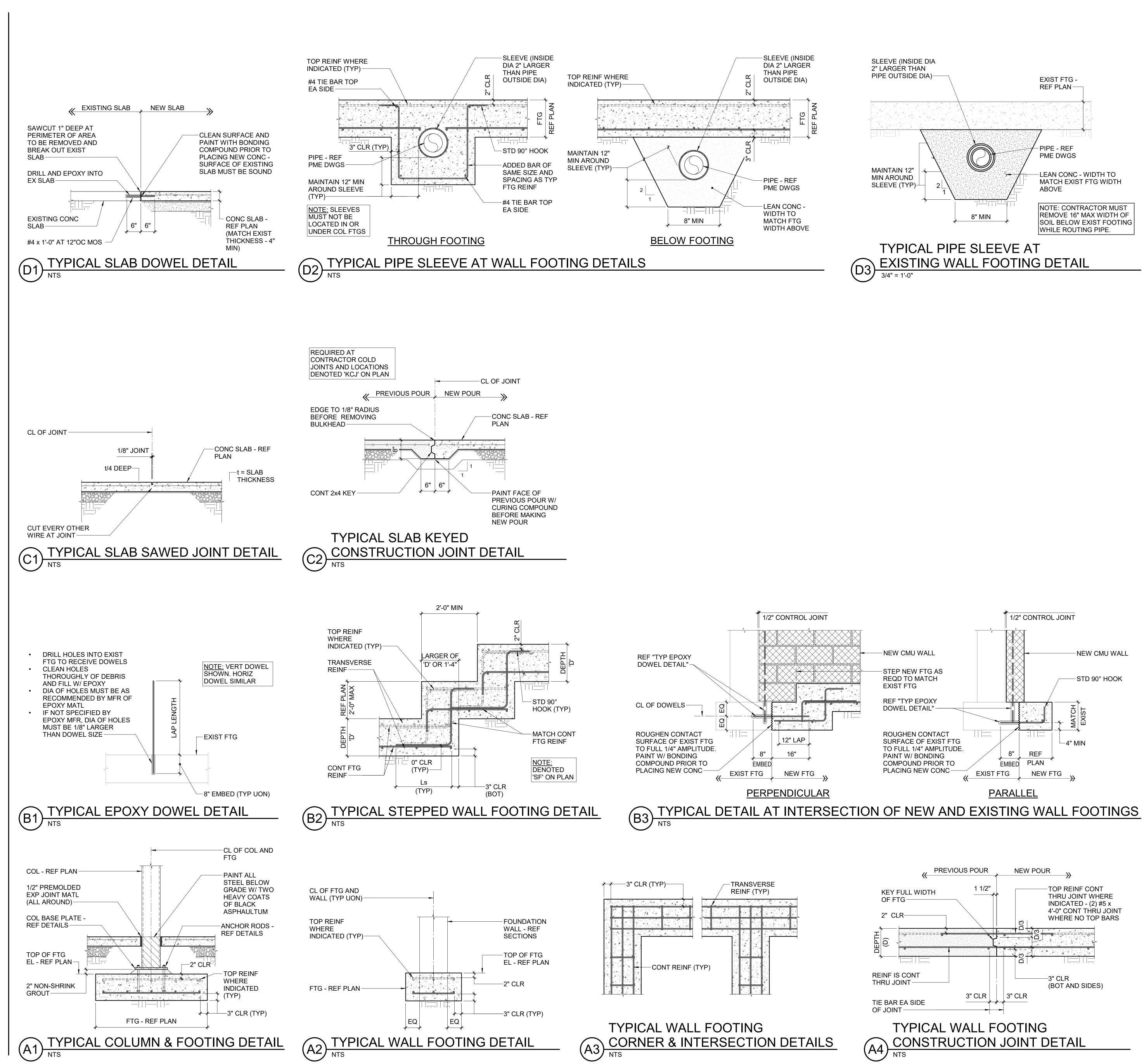
NOTES:

- 1. FIELD LOCATE WITH GPR SCANNING AND MARK ALL REINFORCING (TOP AND BOTTOM) PRIOR TO CORE DRILLING.
- 2. LOCATE CORE HOLE BETWEEN REINFORCING.
- 3. SHIFT CORE HOLE AS REQUIRED TO PROVIDE MINIMUM 1" CLEAR TO ALL REINFORCING.
- 4. REINFORCING MUST NOT BE CUT WITHOUT SPECIFIC APPROVAL OF THE ENGINEER.
- CORE DRILLING MUST NOT BE LOCATED IN BEAMS, JOISTS, OR COLUMNS. 5.
- CORE LOCATIONS ARE NOT LOCATED ON PLAN, CONTRACTOR MUST COORDINATE WITH MECHANICAL, 6 ELECTRICAL, AND PLUMBING DRAWINGS.

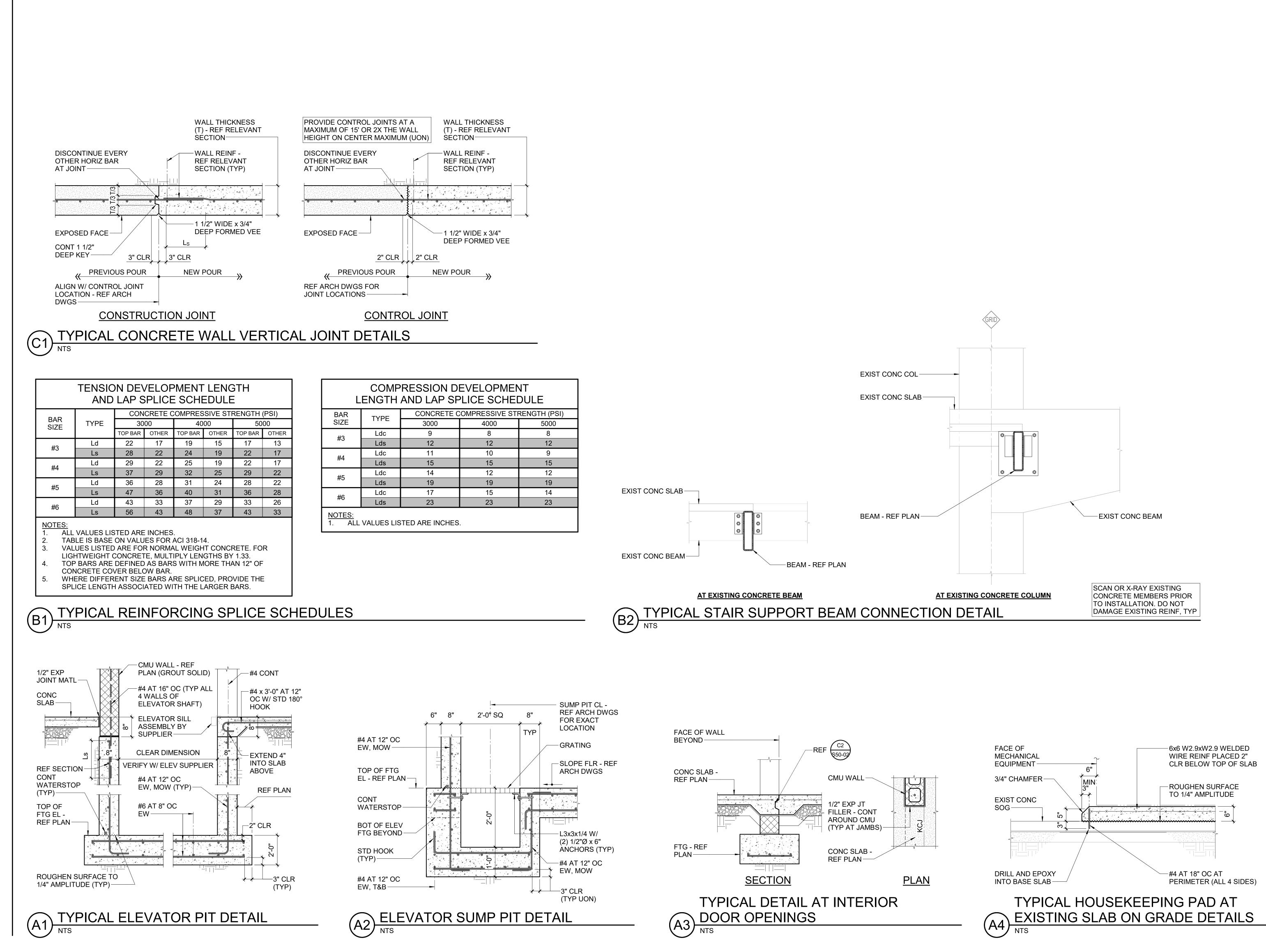
	- SPACING VARIES - CORE DRILL PIPE SLEEVE FIELD VERIFY
EXIST REINF STEEL 1" MIN	1" MIN / 1" MIN
EXIST CONCRETE SLAB CORE DRILLED	TYPICAL REINF BEYOND
HOLE 6"Ø MAX	SECTION
TYP SLAB CORE IN EXIS	TING CONCRETE SLA
NTS NTS	

SLAB DRILL









EXIST CONC SLAB		
EXIST CONC BEAM	BEAM - REF PLAN	BEAM - REF PLAN

N DEVELOPMENT	
SPLICE SCHEDULE	
TE COMPRESSIVE STRENGTH (PSI)	
4000	5000
8	8
12	12
10	9
15	15
12	12
19	19
15	14
23	23
	PLICE SCHE COMPRESSIVE STR 4000 8 12 10 15 12 12 19 15

